

Colegio Dominicano de Ingenieros, Arquitectos y Agrimensores (CODIA)
AÑO DE LA RECUPERACIÓN Y MODERNIZACIÓN DEL CODIA
JUNTA DIRECTIVA NACIONAL 2022- 2023

CONGRESO INTERNACIONAL TERREMOTOS Y VULNERABILIDAD

- SUELOS
- ESTRUCTURAS SISMOS RESISTENTES,
- DIAGNOSTICO POST-SISMO.

Días: 22-23-24-25 Junio 2023

Rep. Dom. 2005

Turquia 2023

Ingenieria Sismica : Evolucion del conocimiento y sus reflejos en las Normativas

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Synopsis

- ❖ Soil Mechanics and Soil Dynamics
- ❖ Main Geotechnical Earthqu. problems
- ❖ Focus on Local Seismic Response
(with Code references)
- ❖ Overview on RSL in Near-Fault conditions

Geotecnica (*Mecanica de suelos*)

Granular materials (solid particles, void full of air and/or water, degree of saturation)

Grain size

Coarse grained soils

Fine grained soils

Permeability

Coarse soils $k = 10^0 \div 10^{-2}$ cm/s

Fine soils $k = 10^{-8} \div 10^{-9}$ cm/s

«SANDY soils»

DRAINED behaviour «D»

«CLAYEY soils»

UNDRAINED behaviour «ND»

CIVIL ENGINEERING problems

Deformation (e.g. foundation settlements)

Failure (safety against collapse)

Dinamica de suelos y Ingenieria Geotecnica Sismica

Dynamic behaviour of soils (small, medium and large deformations ... failure)

Small deformations

– *In situ tests (e.g. DH, CH, MASW)*

Seismic Codes : Vs (or VR measurement)

Medium-Large deformations – Laboratory tests (not common in practise)

*Seismic Codes : no mandatory
(literature curves)*

Dinamica de suelos y Ingenieria Geotecnica Sismica

Dynamic behaviour of soils (small, medium and large deformation ... failure)

Soil – Seismic Wave dynamic interaction

Small-Medium deformations

e.g. Local seismic response

e.g. Foundation Kinematic Interaction

Large deformations

e.g. Liquefaction (sandy soils)

e.g. Landslides, foundation collapse

CIVIL ENGINEERING problems

Deformation (e.g. settlements, lateral displacements)

(PBD : Performance based design !!!)

Dinamica de suelos y Ingenieria Geotecnica Sismica

Soil – Seismic Wave dynamic interaction

Small-Medium deformations

e.g. Local seismic response

INTRODUCTION

Engineering science develops from the observation of natural phenomena that happen

We try to interpret and simulate them by means of physical and mathematical approaches

Hence we have to check if our theoretical approaches are effective, by means of experimental verification

- 
- **OBSERVATION of Natural Phenomena**
 - **THEORETICAL SIMULATION**
 - **EXPERIMENTAL VERIFICATION**



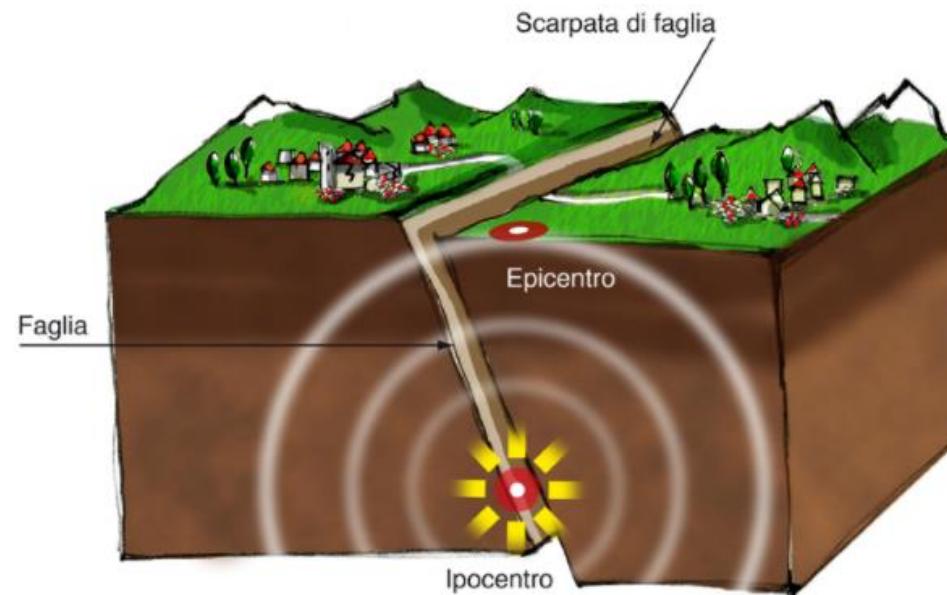
Traditional well studied Earthquakes : **FAR-FAULT Earthquakes (FF)**

Earthquake phenomenon

Deep tectonic plates tends to move relatively : High stress accumulation !

When the stresses reach a «limit» value, a failure phenomenon takes place.

The released energy propagates as seismic waves, from the focus all over around, even towards the free surface: **EARTHQUAKE !!!**

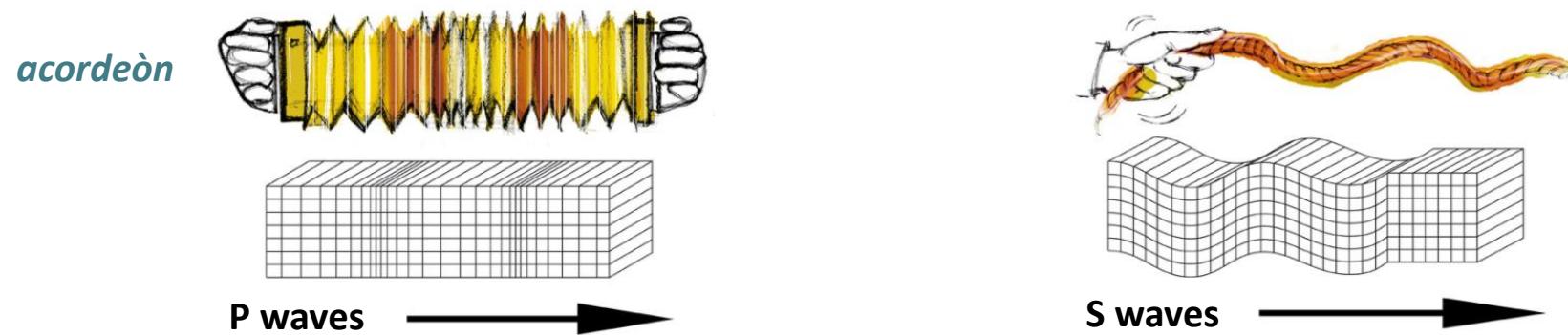


The source fault is sometimes visible on the surface.

Seismic waves propagation body waves

Onde P *Primae*: vibration in the direction of propagation

Onde S *Secundae*: vibration in the direction orthogonal to the direction of propagation

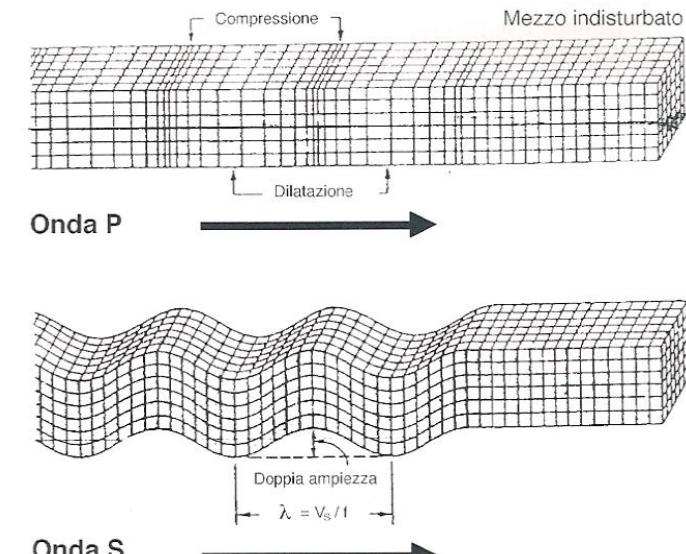


- P waves are faster than S waves (about 1.7 times).
- When the P and S waves reach the surface, they generate Superficial waves.

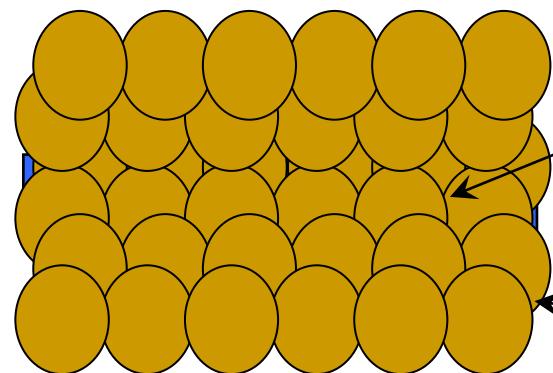
P and S waves

- Earthquake and seismic waves
- Moto sussultorio/ondulatorio
- **S waves and shear stiffness Go**

➤ Measurement methods
(geophysical and geotechnical)



➤ Saturated soil



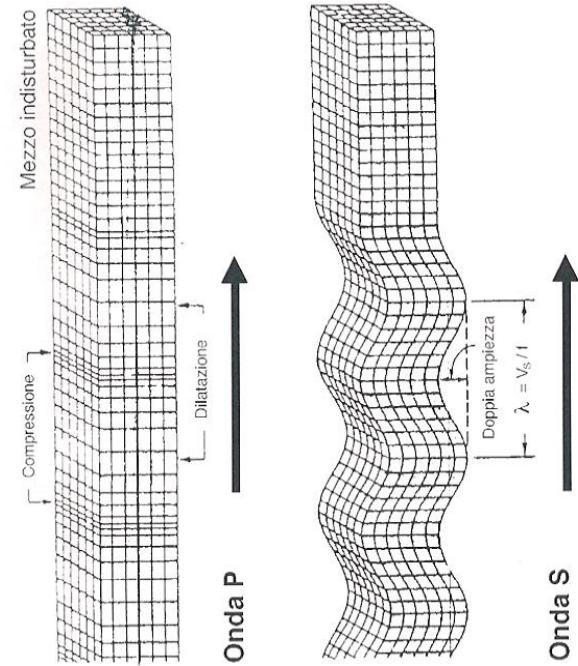
Pore fluid
incompressible

Soil skeleton

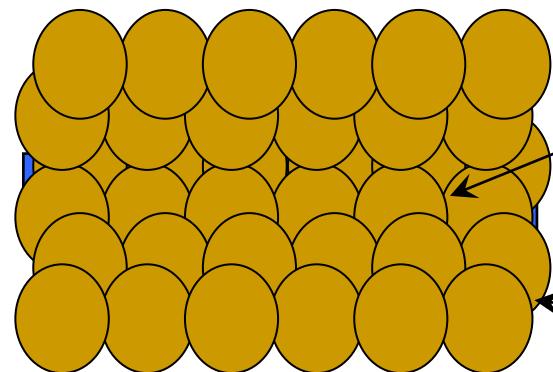
P and S waves

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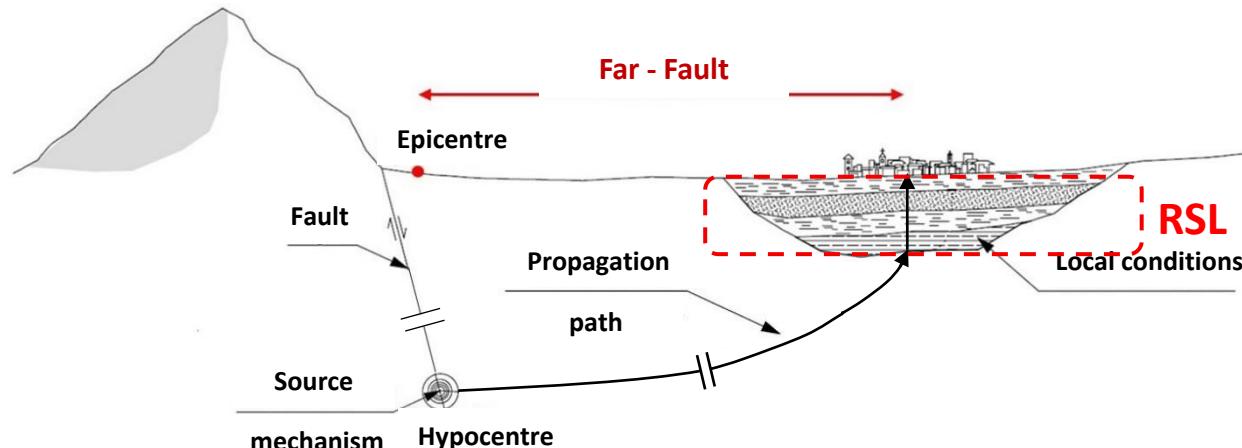
Pore fluid
incompressible

Soil skeleton

FAR-FAULT condition

F
A
R

F
I
E
L
D

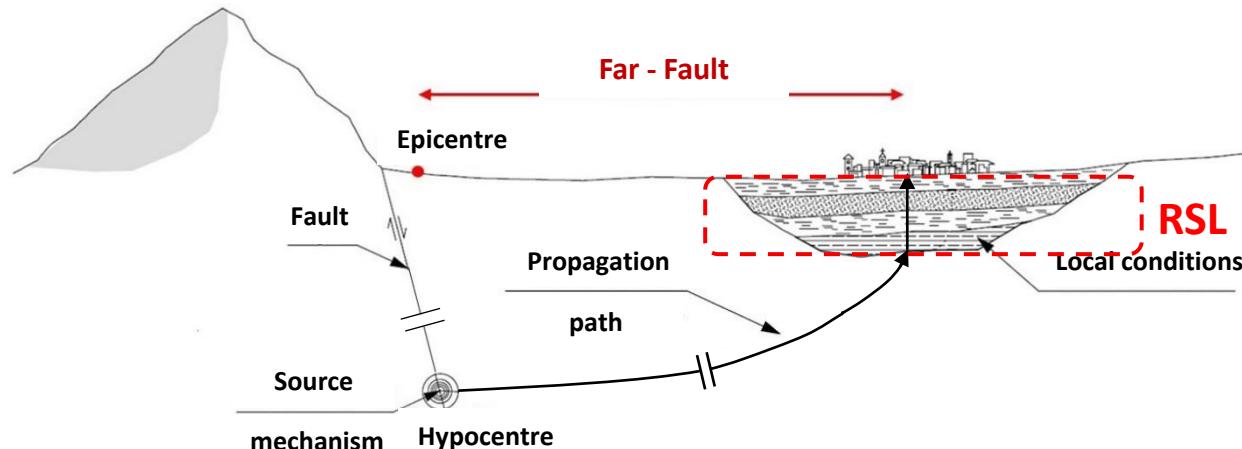


- ❖ P and S waves emerge on the surface with vertical propagation direction
- ❖ P waves arrive first (vertical earthquake), S waves arrive later (undulatory earthquake)
- ❖ **Vertical accelerations rather smaller than the horizontal ones**
- ❖ Main frequency content - P waves: 5-10 Hz , S waves: 2-5 Hz

FAR-FAULT condition

F
A
R

F
I
E
L
D



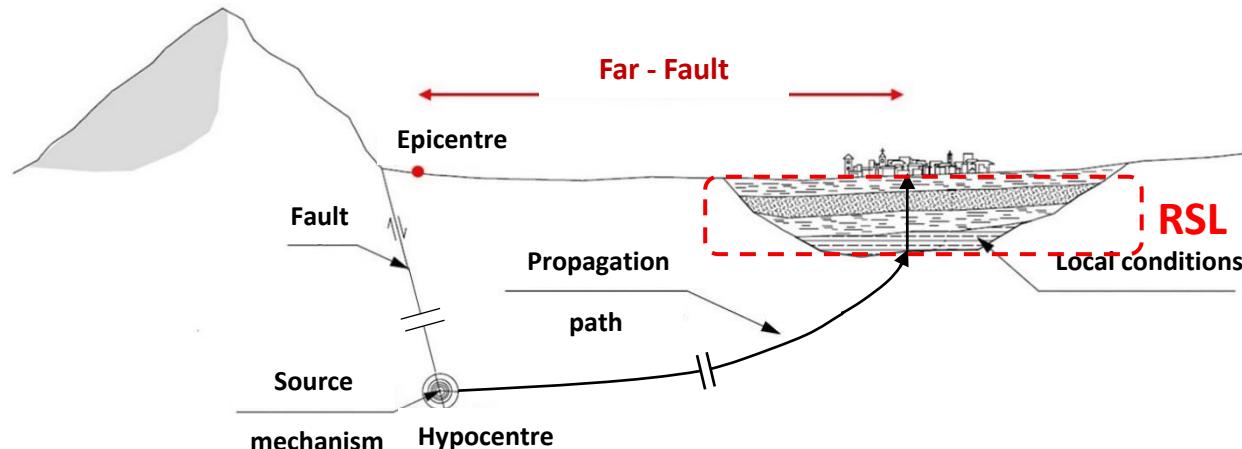
In traditional seismic engineering studies, therefore, the earthquake is schematized as:

- ❖ vertical propagation of only S waves
- ❖ the soils are essentially characterized by their G shear stiffness (and horizontal period T)
- ❖ the actions on the structures are exclusively horizontal actions

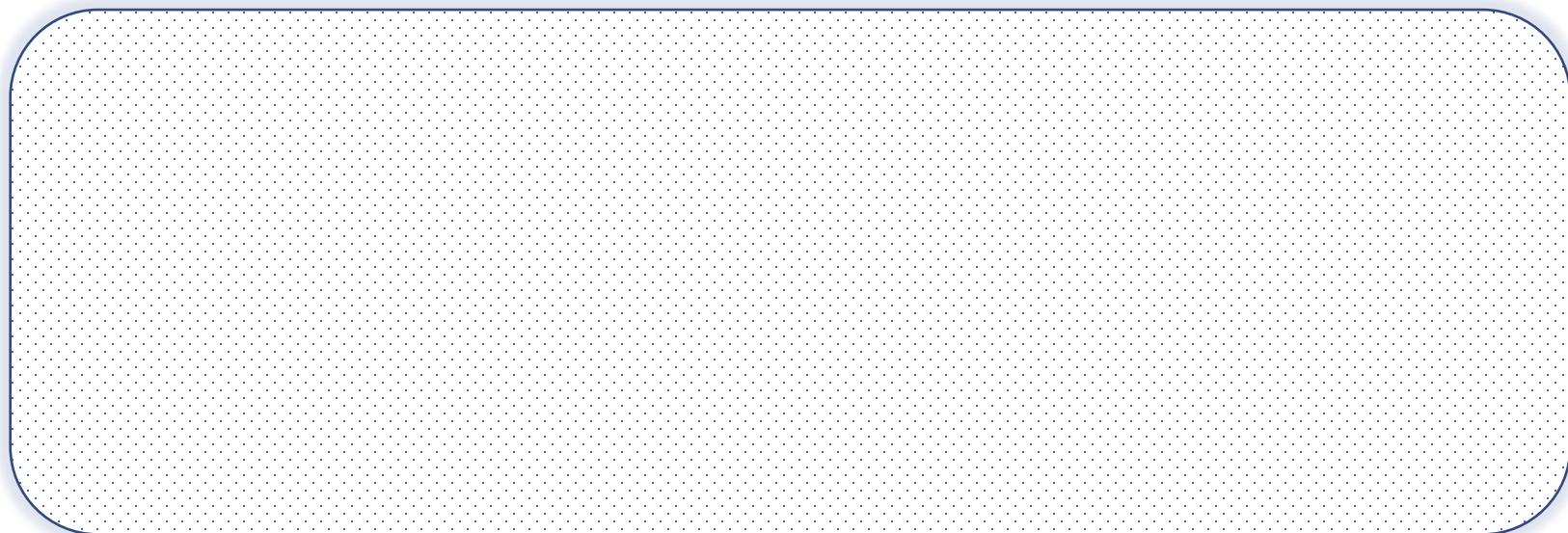
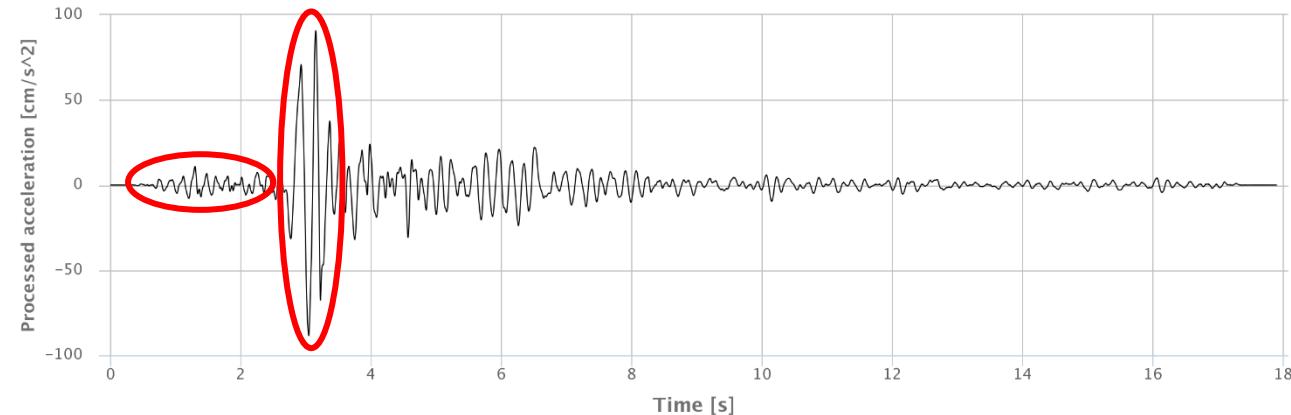
FAR-FAULT condition

F
A
R

F
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D



Seismic Code: This is the kind of earthquake that, up to now, is accomplished
in all the National and International Codes !

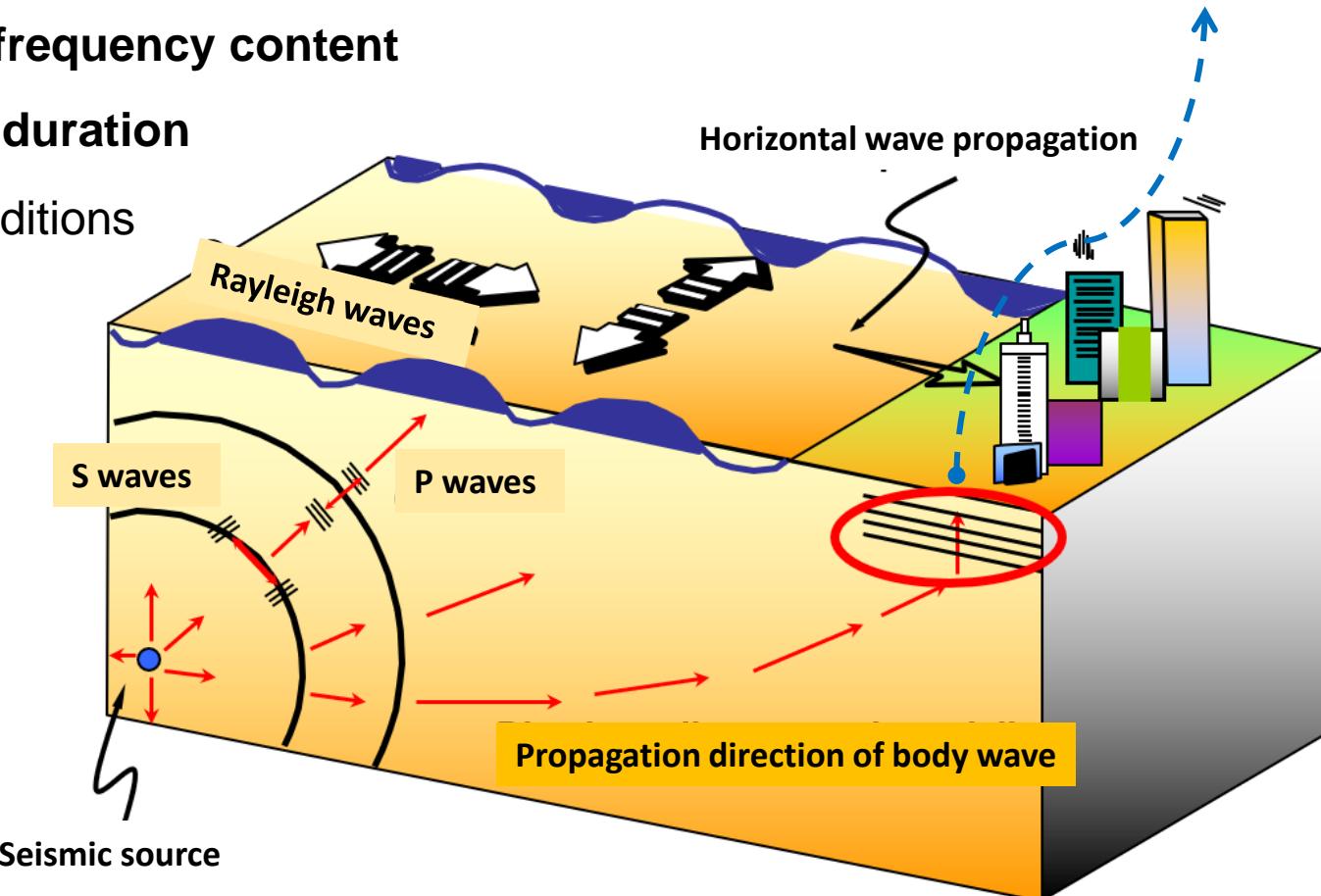
Assisi Earthquake, 1997

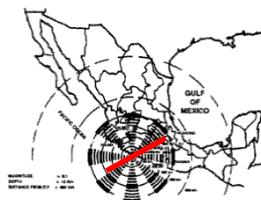
Local Seismic Response (*always in Far-Fault conditions*)

- Modification of :
- amplitude
 - frequency content
 - duration

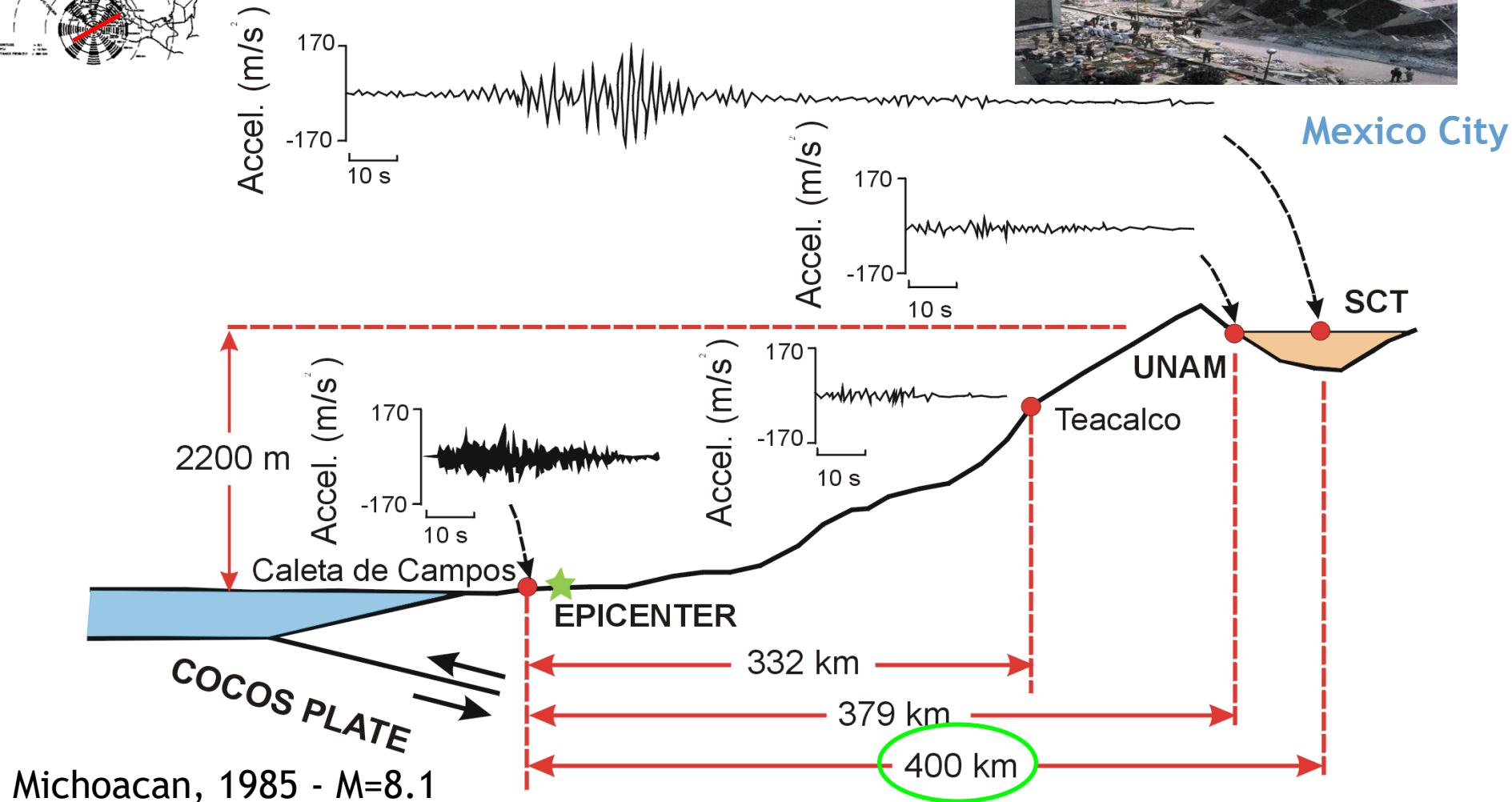
due to local soil conditions

Microzonation





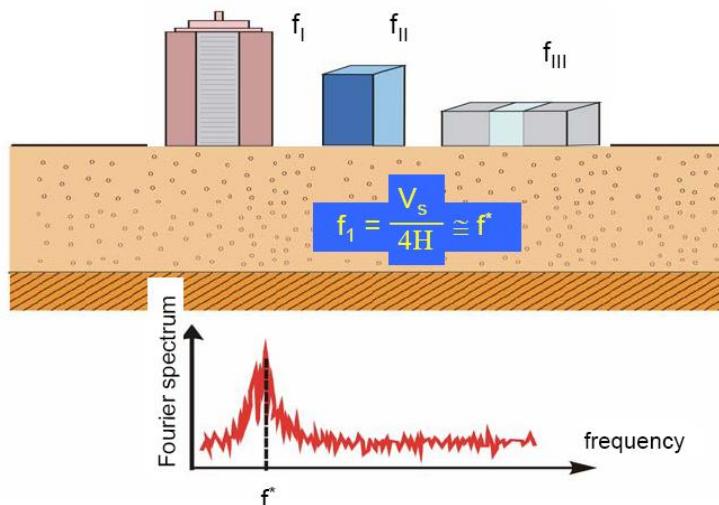
LOCAL SEISMIC RESPONSE : Experimental evidence



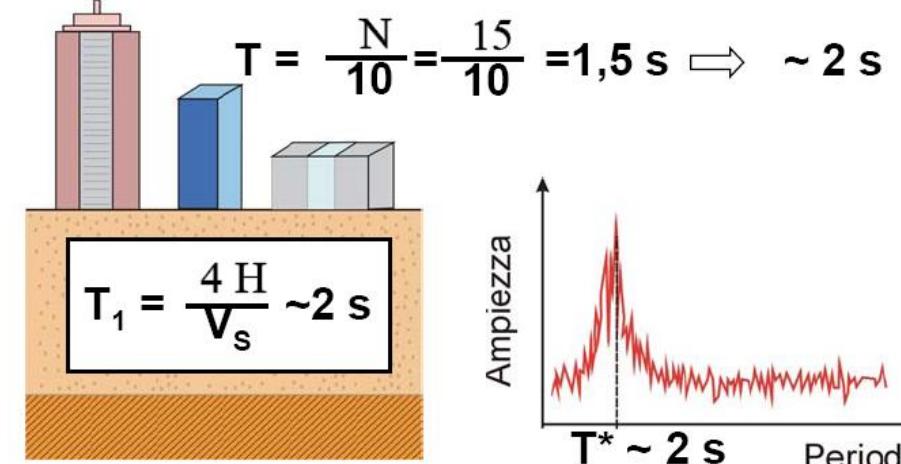
RESONANCE Phenomena

Seismic waves - soil - structure Resonance

Scenario più pericoloso:doppia risonanza



Città del Messico:doppia risonanza



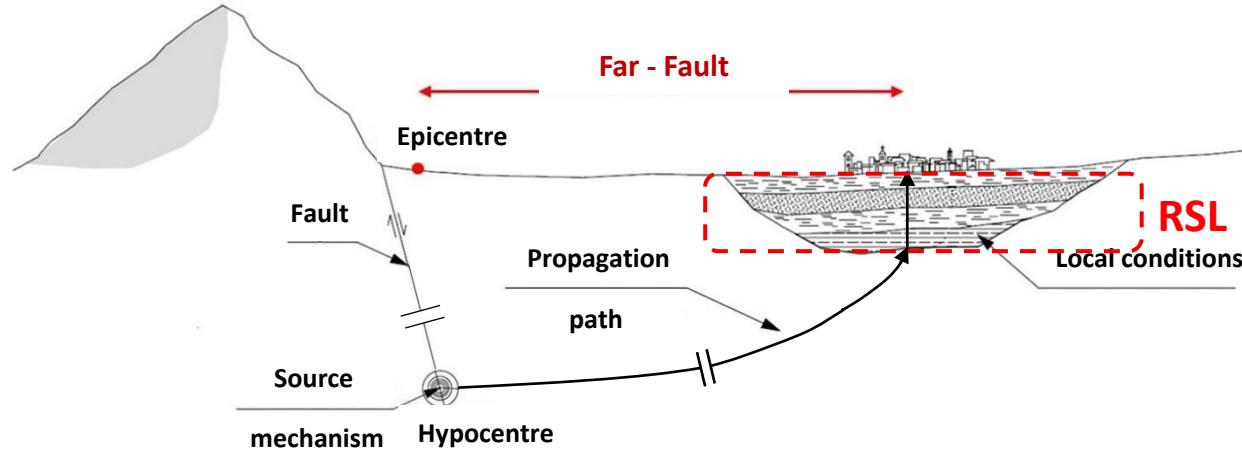
Coincidenza tra periodo caratteristico
dell'accelerogramma, periodo fondamentale del sito e
periodo di vibrazione della struttura

Seismic Code: “T” is always the main horizontal period of the structure !

SEISMIC CODES: Evaluation of the Local Seismic Response

F
A
R

F
I
E
L
D

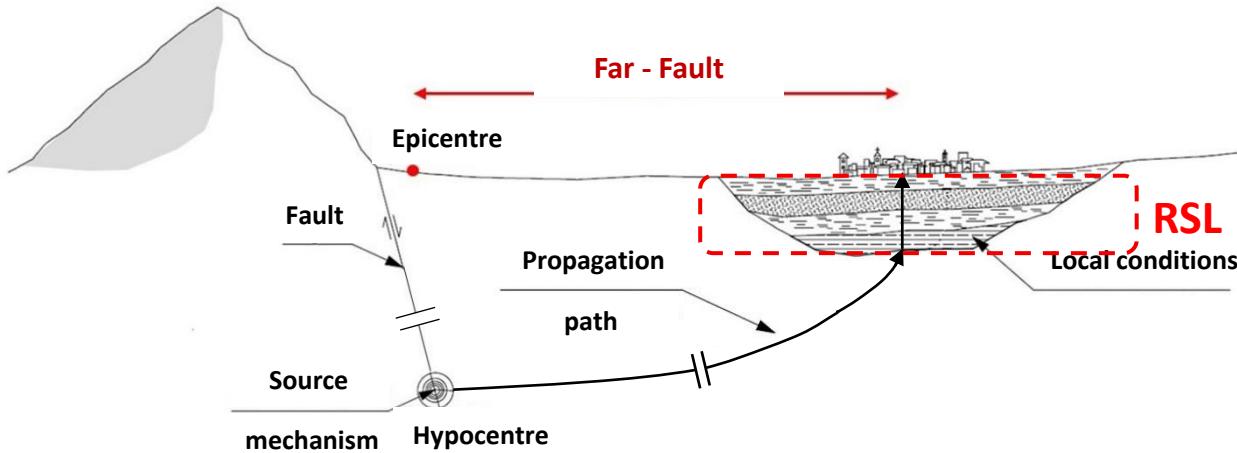


- True RSL** – Analysis of propagation of vertical SH waves, coming up from the bedrock (or a stiff soil with $V_s \geq 800$ m/s)
- Simplified RSL** - Amplification factors S_s for different soil categories (5+2), based on subsoil equivalent shear wave velocity

SEISMIC CODES: Evaluation of the Local Seismic Response

F
A
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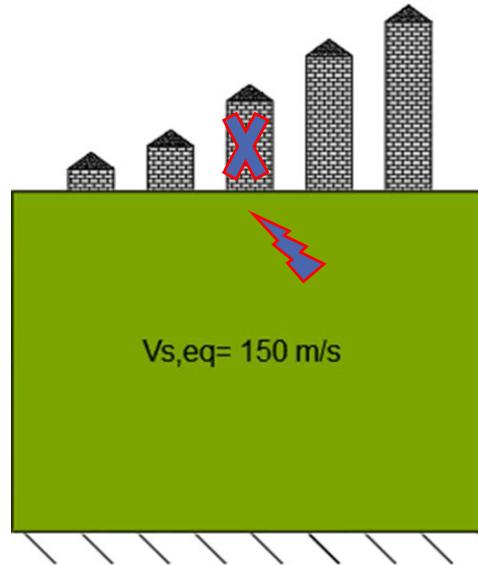
Simplified RSL : Two main observations

FIRST - Soil categories should not be based on $V_{S,30}$ but on $V_{S,eq}$ when the bedrock is within the first 30 m (see NTC2018)

SECOND - Soil categories are valid only for deposits with stiffness constant or increasing with depth (NEGLECTED IN THE PRACTICE)!!!

Effect of Soil Stiffness sequence for three subsoils having the same $V_{s,30}$

$$V_{s,eq} = \frac{H}{\sum \frac{H_i}{V_{s,i}}} = 150 \frac{m}{s}$$



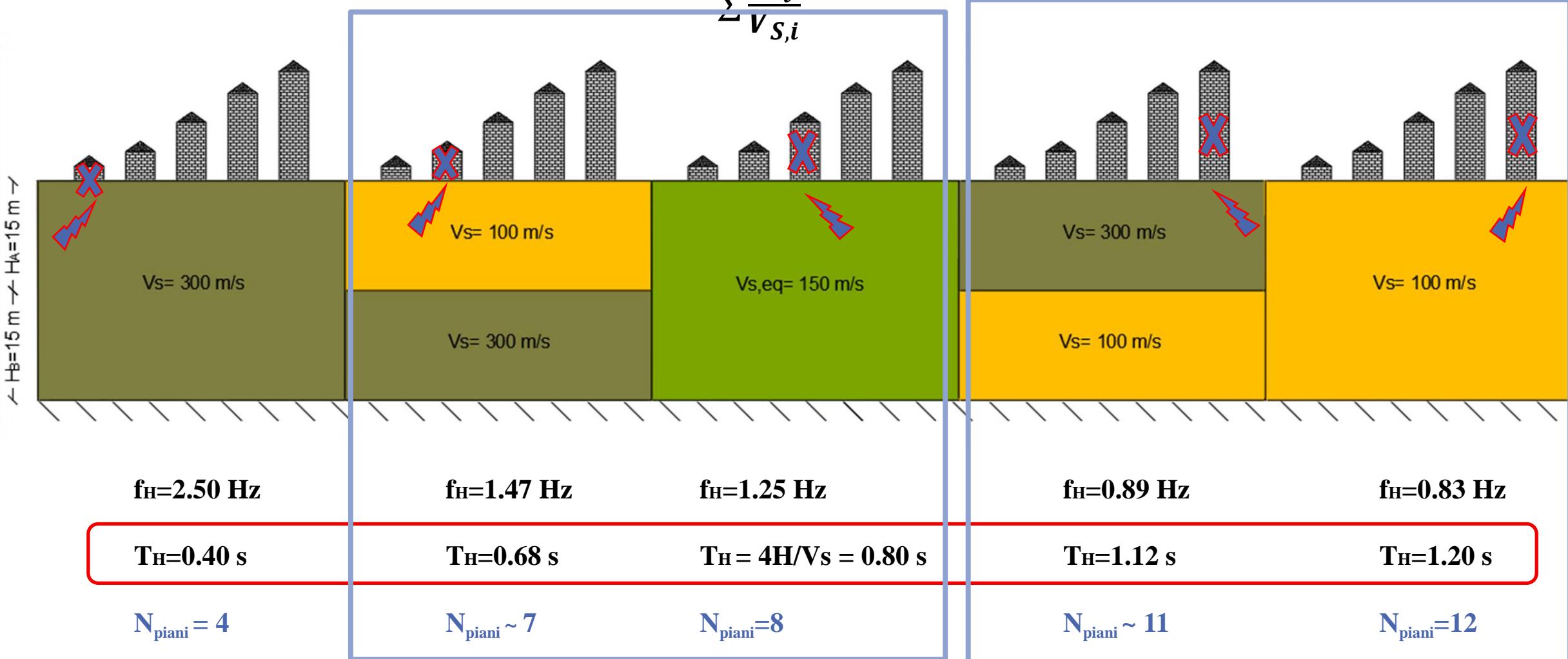
$$f_H = 1.25 \text{ Hz}$$

$$T_H = 4H/V_s = 0.80 \text{ s}$$

$$N_{piani} = 8$$

Effect of Soil Stiffness sequence for three subsoils having the same $V_{s,30}$

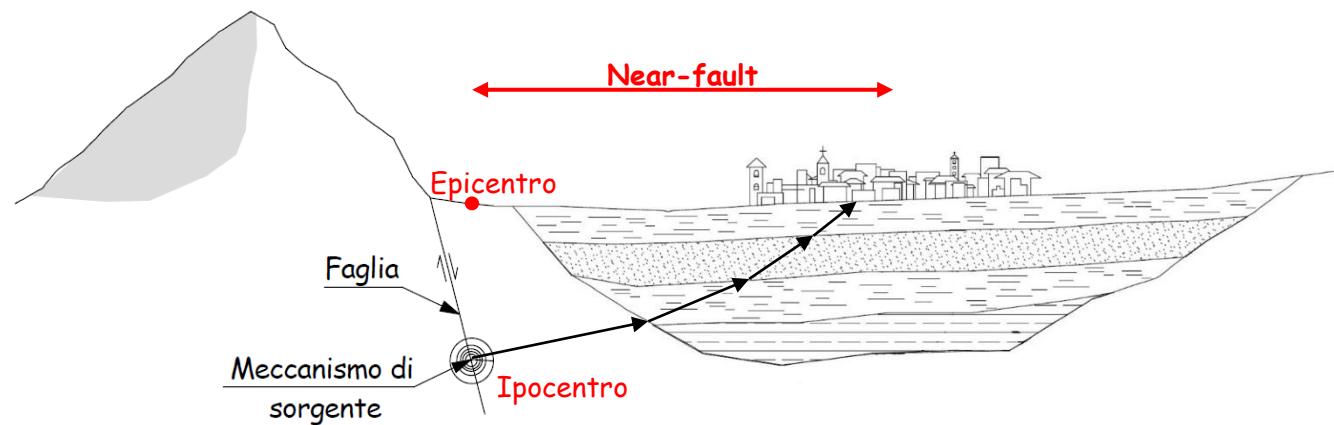
$$V_{s,eq} = \frac{H}{\sum \frac{H_i}{V_{s,i}}} = 150 \frac{m}{s}$$



Traditional well studied Earthquakes,
accounted for in national and international seismic codes :
FAR-FAULT Earthquakes (FF)

What happens in

NEAR-FAULT condition ???



INTRODUCTION

Engineering science develops from the observation of natural phenomena that happen

We try to interpret and simulate them by means of physical and mathematical approaches

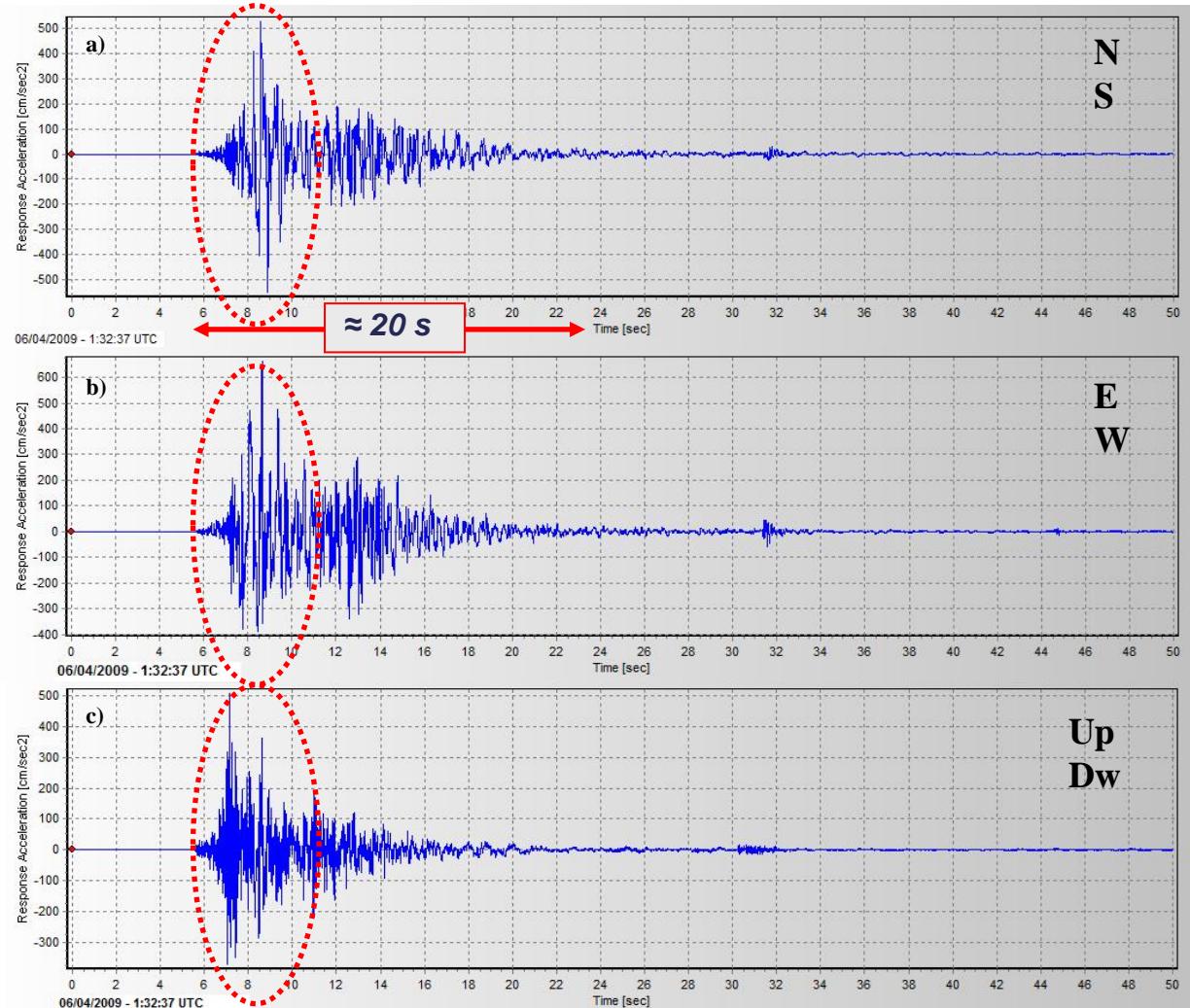
Hence we have to check if our theoretical approaches are effective, by means of experimental verification

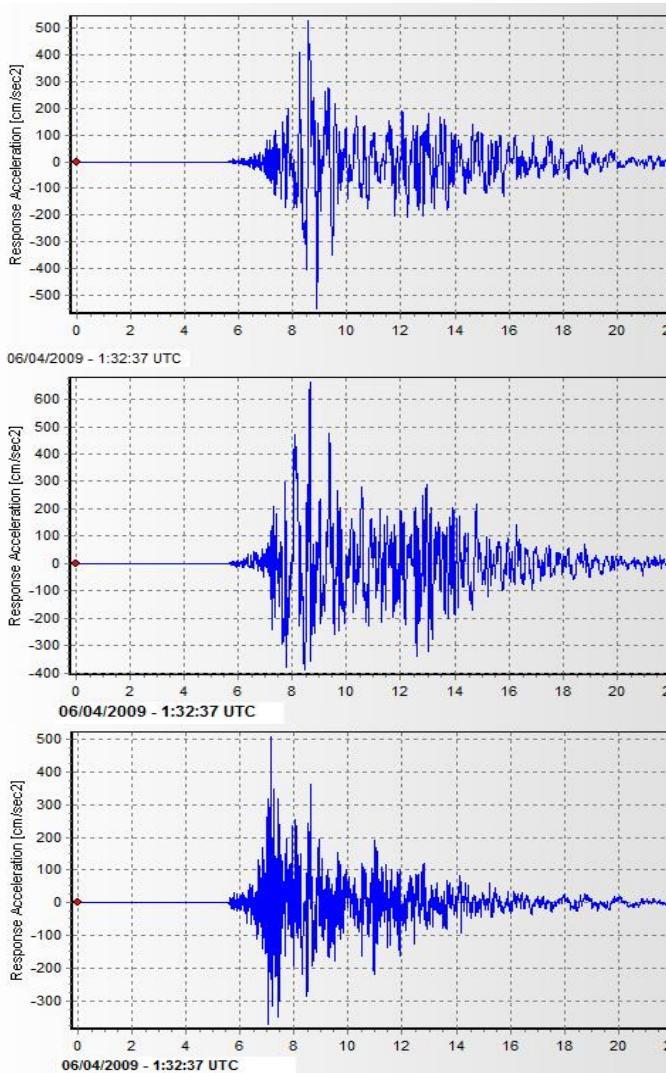
- 
- **OBSERVATION of Natural Phenomena**
 - **THEORETICAL SIMULATION**
 - **EXPERIMENTAL VERIFICATION**

L'Aquila, 2009 April 6



L'Aquila Earthquake, 2009: Time Histories of acceleration at AQV (42.377N;13.344E)
Station epicenter distance 4.8 km





April 6, 2009 L'Aquila earthquake

NEAR-FAULT EARTHQUAKES

STRUCTURAL DAMAGE



2009 L'Aquila Earthquake

Examples of Damage to due to
high vertical component of ground motion



L'Aquila 2009 – Residential buildings with damage due to high vertical seismic actions

2009 L'Aquila Earthquake

Examples of Damage to due to
high vertical component of ground motion



L'Aquila 2009 – Residential buildings with damage to bow-windows under high vertical seismic actions

2009 L'Aquila Earthquake

Examples of Damage to due to
high vertical component of ground motion



L'Aquila 2009 – Residential buildings with damage to intermediate floors because of their higher frequency vibration modes

Columns

Failure at column top section (circular & rectangular).

Stirrups spacing (certainly!) greater than 200mm.

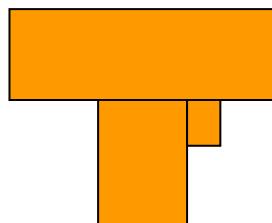
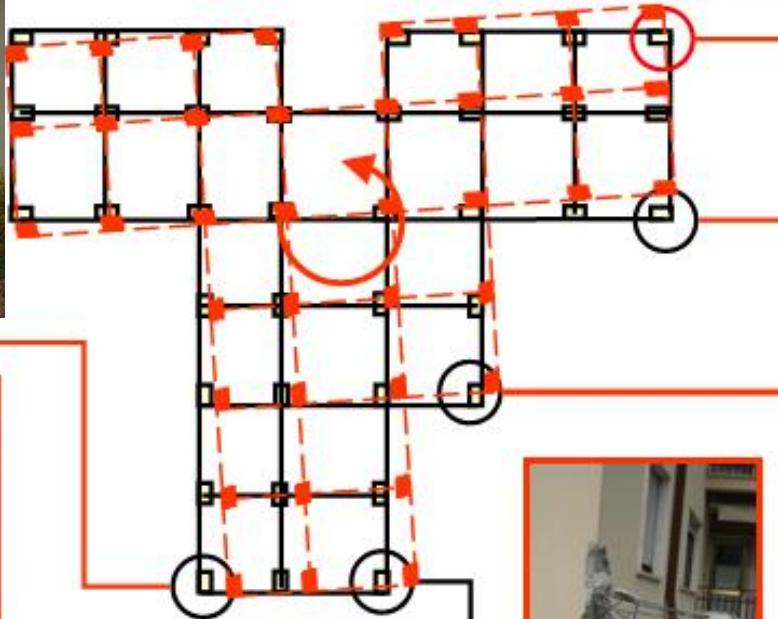
Inadequate concrete mix and compression strength

2009 L'Aquila Earthquake



Examples of Damage due to high vertical component of ground motion

Pettino Building : System Global-Collapse



2009 L'Aquila Earthquake

2009 L'Aquila Earthquake



Pettino Building : System Global-Collapse

2016-17 Italia Centrale Earthquake - Accumoli



Examples of Damage
due to
high vertical component
of ground motion

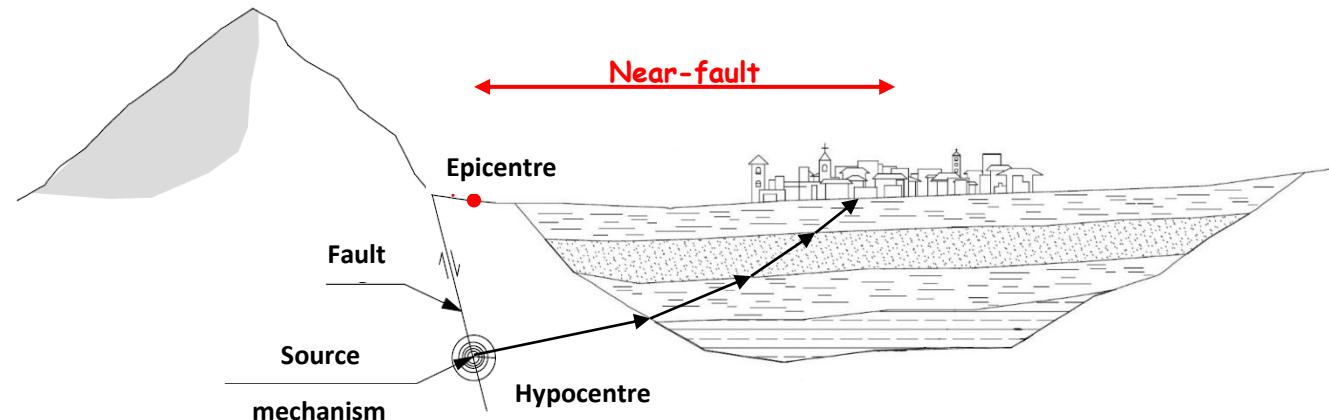
2016-17 Italia Centrale Earthquake - Accumoli

2016-17 Italia Centrale Earthquake - Amatrice

2016-17 Italia Centrale Earthquake - Amatrice

NEAR-FAULT condition

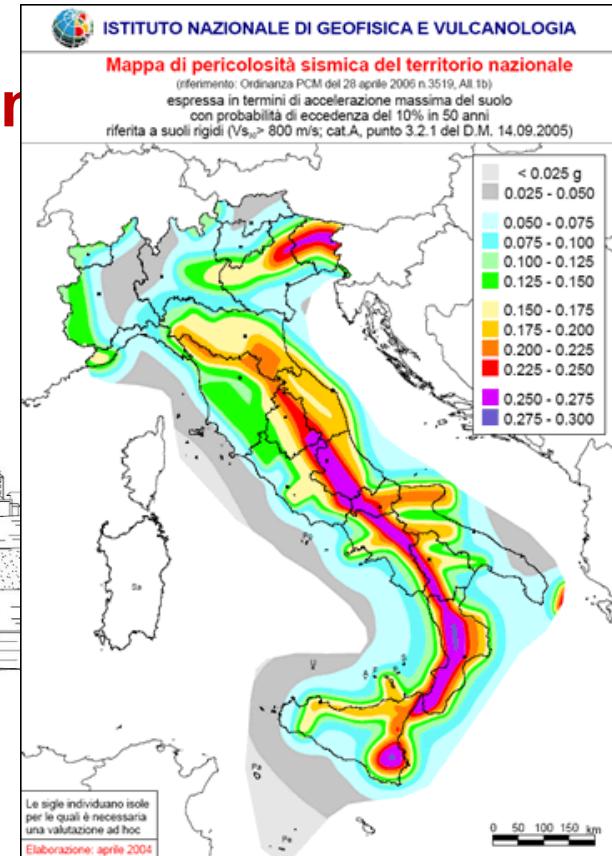
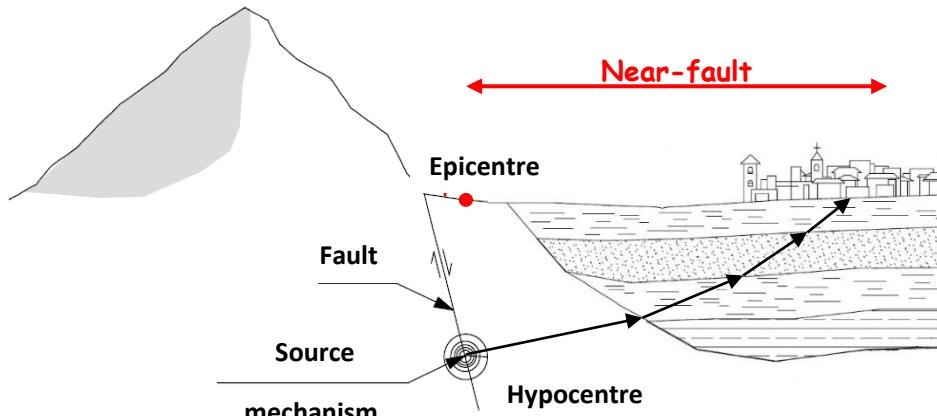
NEAR
F A U L T



- ❖ P and S emerge on the surface with inclined direction → contribute to both horizontal and vertical oscillations
- ❖ P waves and S waves arrive almost simultaneously → combined motion
- ❖ Comparable values of vertical and horizontal accelerations
- ❖ Frequency content of horizontal and vertical motions are similar (from low to higher frequencies)

NEAR-FAULT conditions

NEAR
FAULT



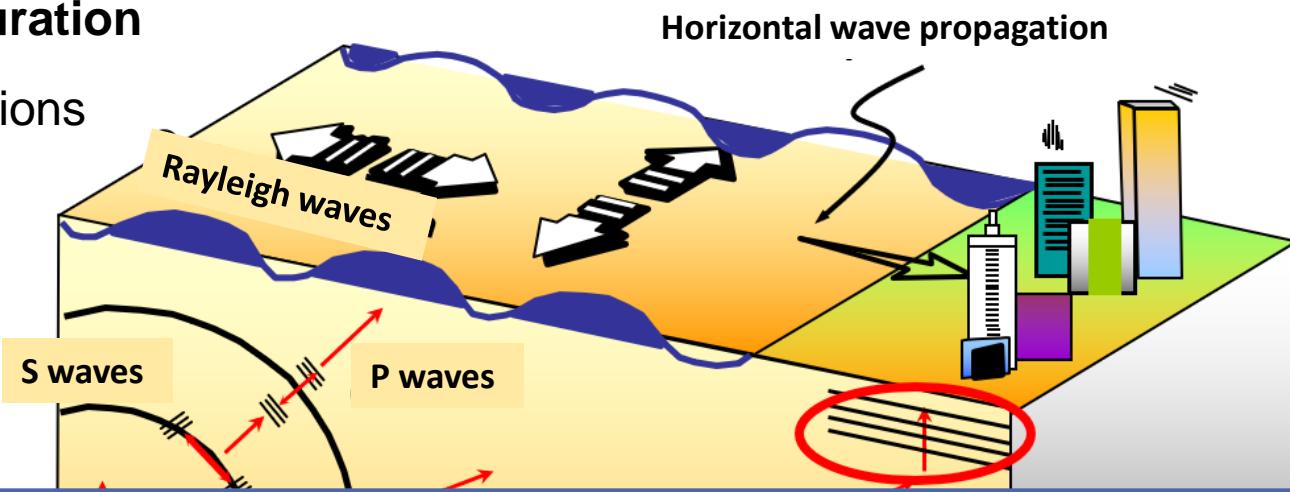
It is evident, therefore, that the traditional approach of seismic engineering,
as propagation of S waves only in the vertical direction
(hence only horizontal motion),
is very far from what occurs in near-fault conditions

Local Seismic Response (*always in Far-Fault conditions*)

Modification of :

- amplitude
- frequency content
- duration

due to local soil conditions



What happens in Near-Fault Conditions ?

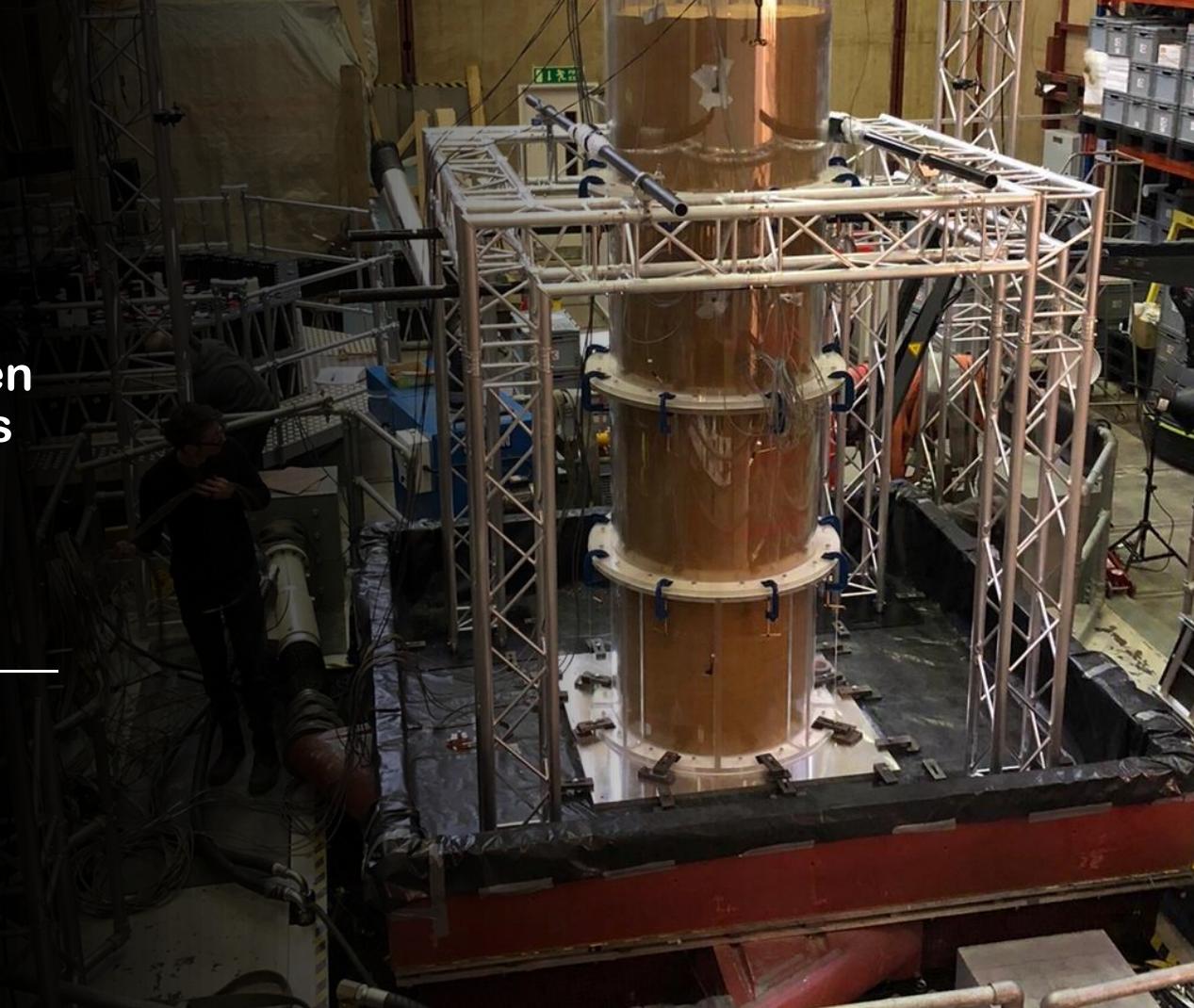
(Soil response to Vertical Accelerations too ... vertical period T_V and Amplification)



Conferencia: Lecciones sobre terremotos recientes en áreas cercanas a fallas

Lessons from
recent earthquakes in
near-fault areas

Armando L. Simonelli



SEISMOLOGY AND EARTHQUAKE ENGINEERING RESEARCH INFRASTRUCTURE ALLIANCE FOR EUROPE SERA



Seismology and
Earthquake Engineering
Research Alliance
for Europe

SHAking Table Testing for Near Fault Effect Evaluation (**SHATTENFEE**)



Lead User

Prof. [Armando Lucio Simonelli](#)

University of Sannio (Italy)
School of Civil Engineering
Department of Engineering



Project SHATTENFEE

PROPOSING RESEARCH TEAM & HOSTING TA FACILITY

PROPOSING RESEARCH TEAM

- University of Sannio – Italy
- National Technical University of Athens - Greece
- University of Trento - Italy
- Norwegian Geotechnical Institute - Norway
- University of Lisbon – Portugal
- University of the West of England



NGI



HOSTING TA FACILITY

BLADE
Bristol Laboratory
for Advanced
Dynamic Engineering
University of Bristol
UK

SERIES PROJECT "PILESI", 2009



Project SHATTENFEE

PROPOSING RESEARCH TEAM & HOSTING TA FACILITY

**PROPOSING
RESEARCH
TEAM**

- University of Sannio – Italy

Armando Lucio Simonelli

Augusto Penna

Luigi Di Sarno



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U
LISBOA
UNIVERSIDADE DE LISBOA

**UWE
Bristol** | University
of the
West of
England

**HOSTING
TA FACILITY**

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SERIES PROJECT "PILESI", 2009



Project SHATTENFEE - Summary

Recent surveys conducted after destructive earthquakes demonstrated that, in near fault (NF) conditions, combined vertical and horizontal motions caused unusual and poorly understood damage to geotechnical and structural systems.

In Near Fault areas earthquake ground motions are significantly:

- *the short distance from the fault;*
- *the seismic waves' incidence direction (generally vertical);*

As a result:

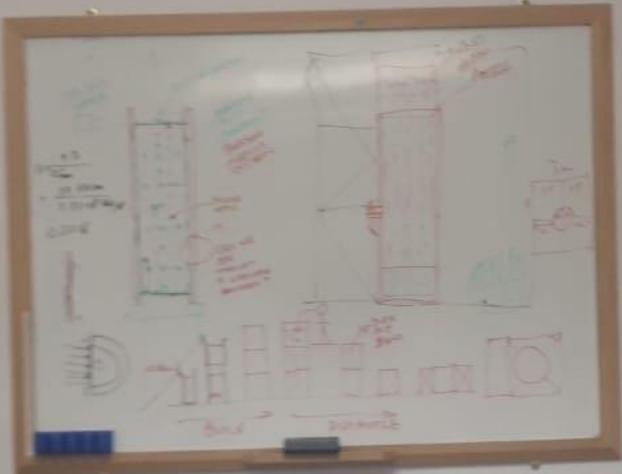
- *the time delay between components of motion is small;*
- *horizontal and vertical motions have similar frequency contents, because they travel along the same path;*
- *horizontal and vertical motions have approximately the same magnitude;*
- *seismic waves with vertical input motion tend to exceed the values provided by standard codes.*

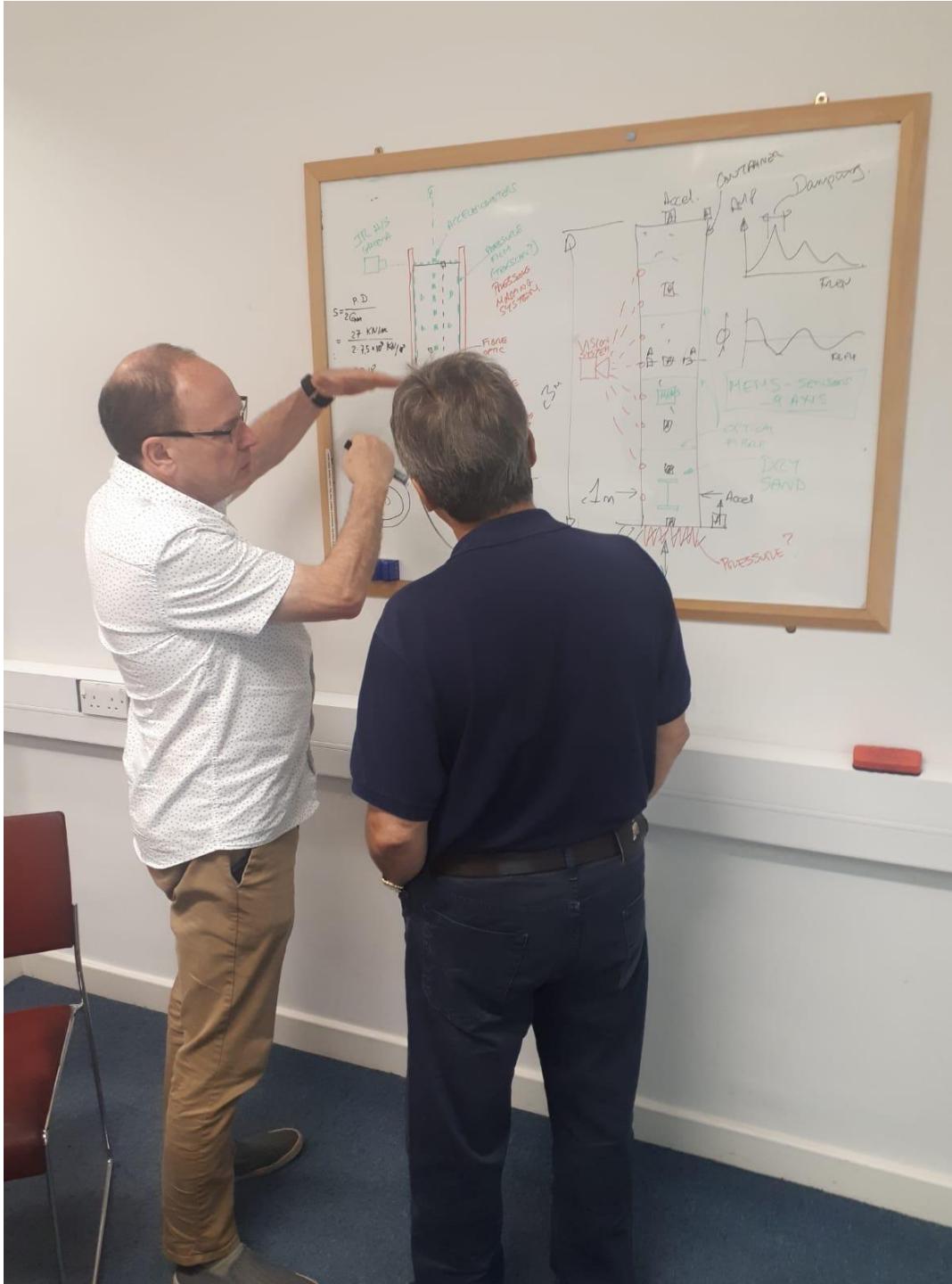
Little is known about the dynamic soil response to vertical motion, particularly its effects on SSI systems, so this project will focus on this aspect of the overall problem.

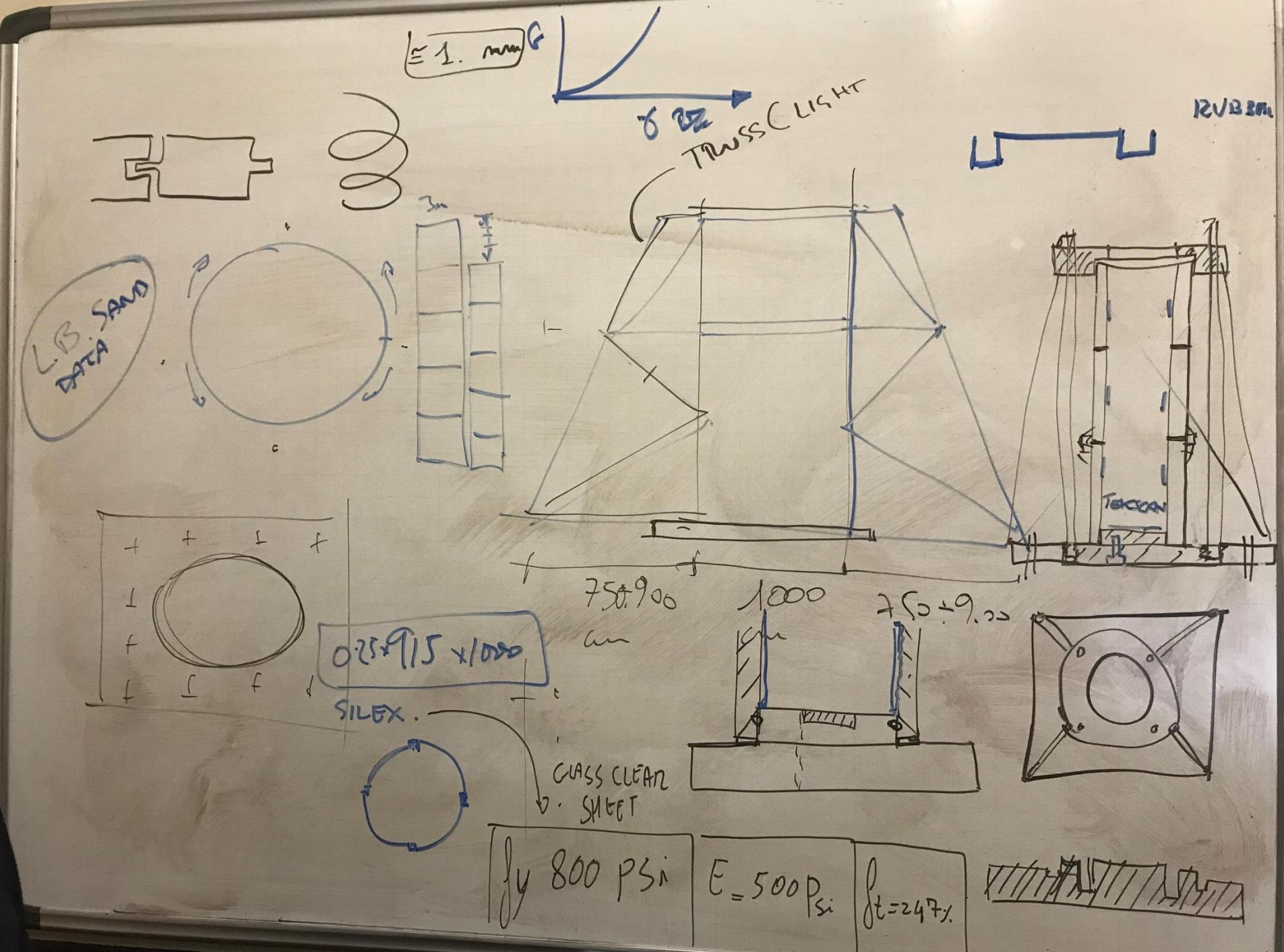
The vertical motion characteristics of a typical soil deposit, with and without the presence of a foundation system and a single-degree-of-freedom oscillator, will be explored experimentally on the 6-DoF shaking table at Bristol University using a new soil container specifically designed for vertical wave propagation.

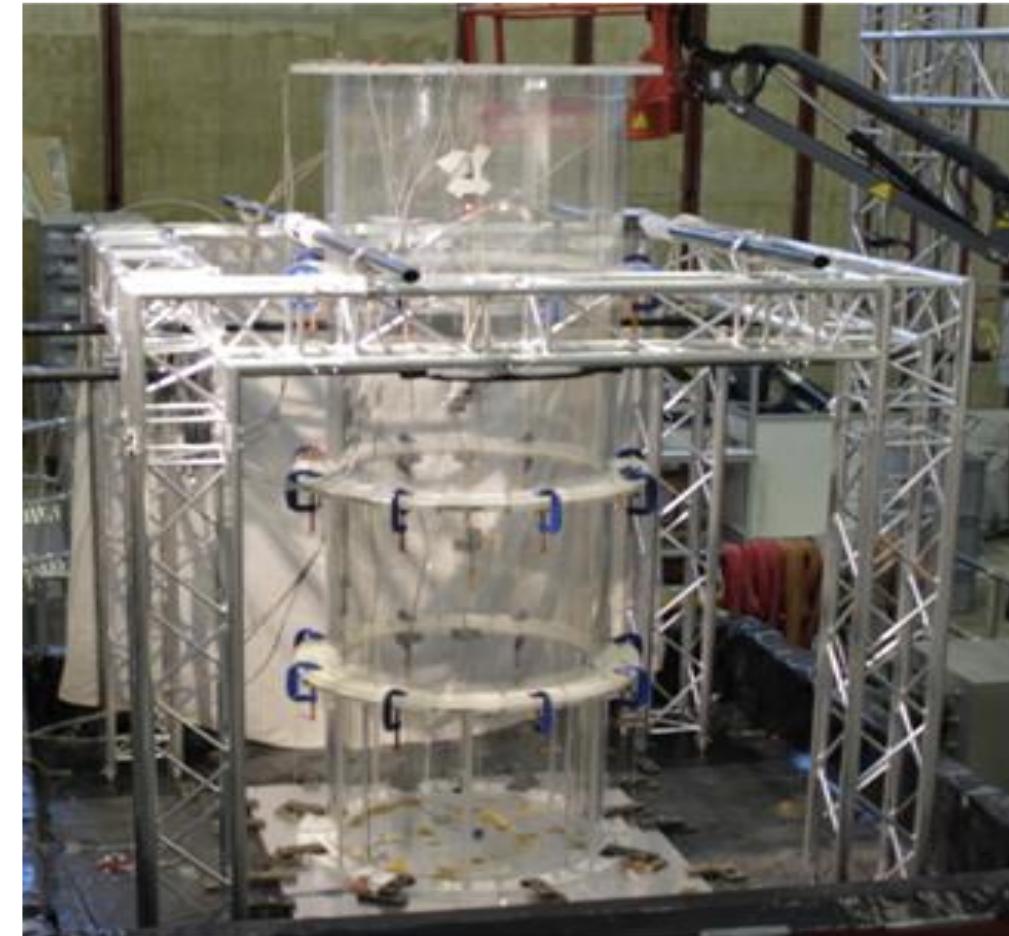
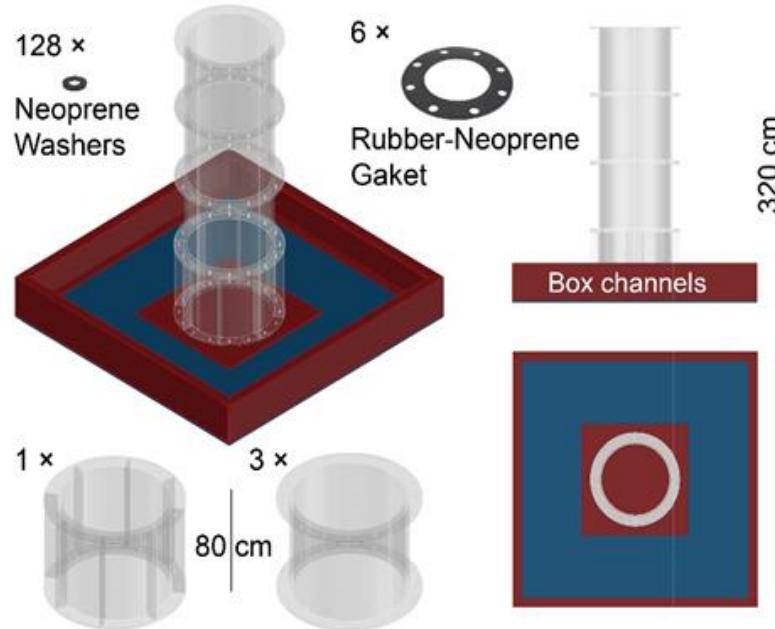
In particular, the vertical dynamic response characteristics, e.g. the period of vertical vibration, damping coefficient, and effect of soil non-linearity and excitation characteristics (e.g., harmonic, impulse, seismic), will be investigated experimentally and numerically to provide fundamental insight and guide further research and code of practice development.

Dynamic identification of soil response under vertical input motion

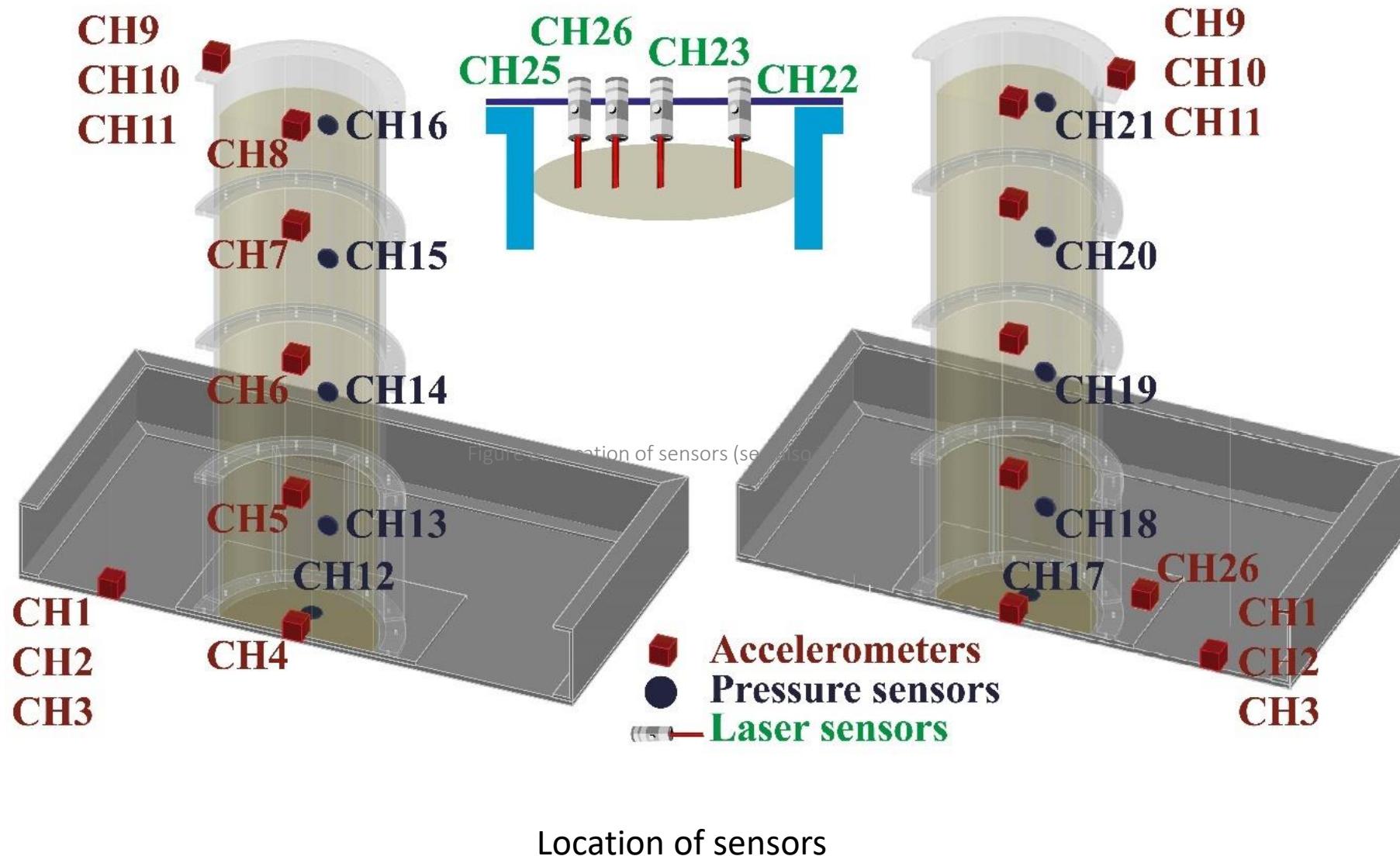


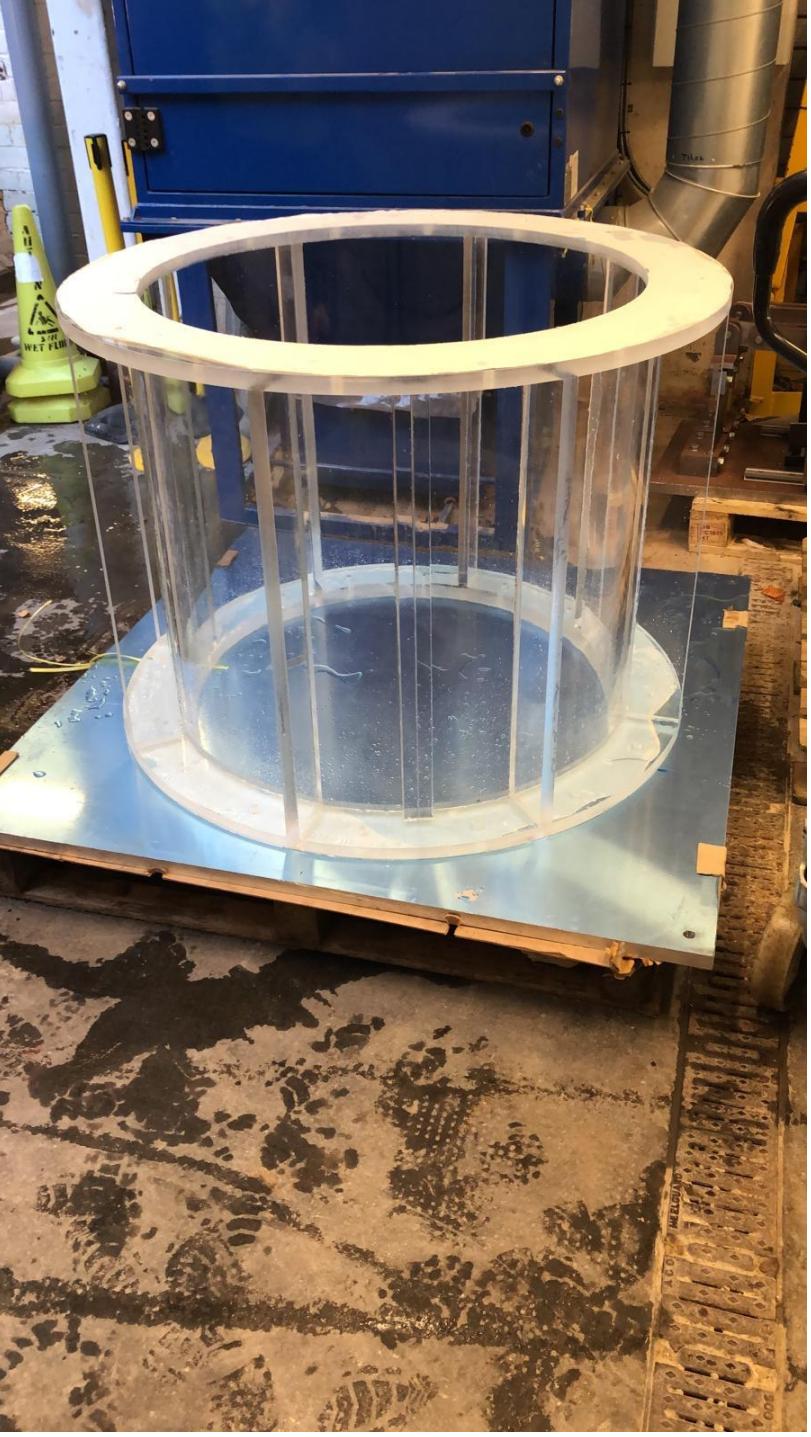




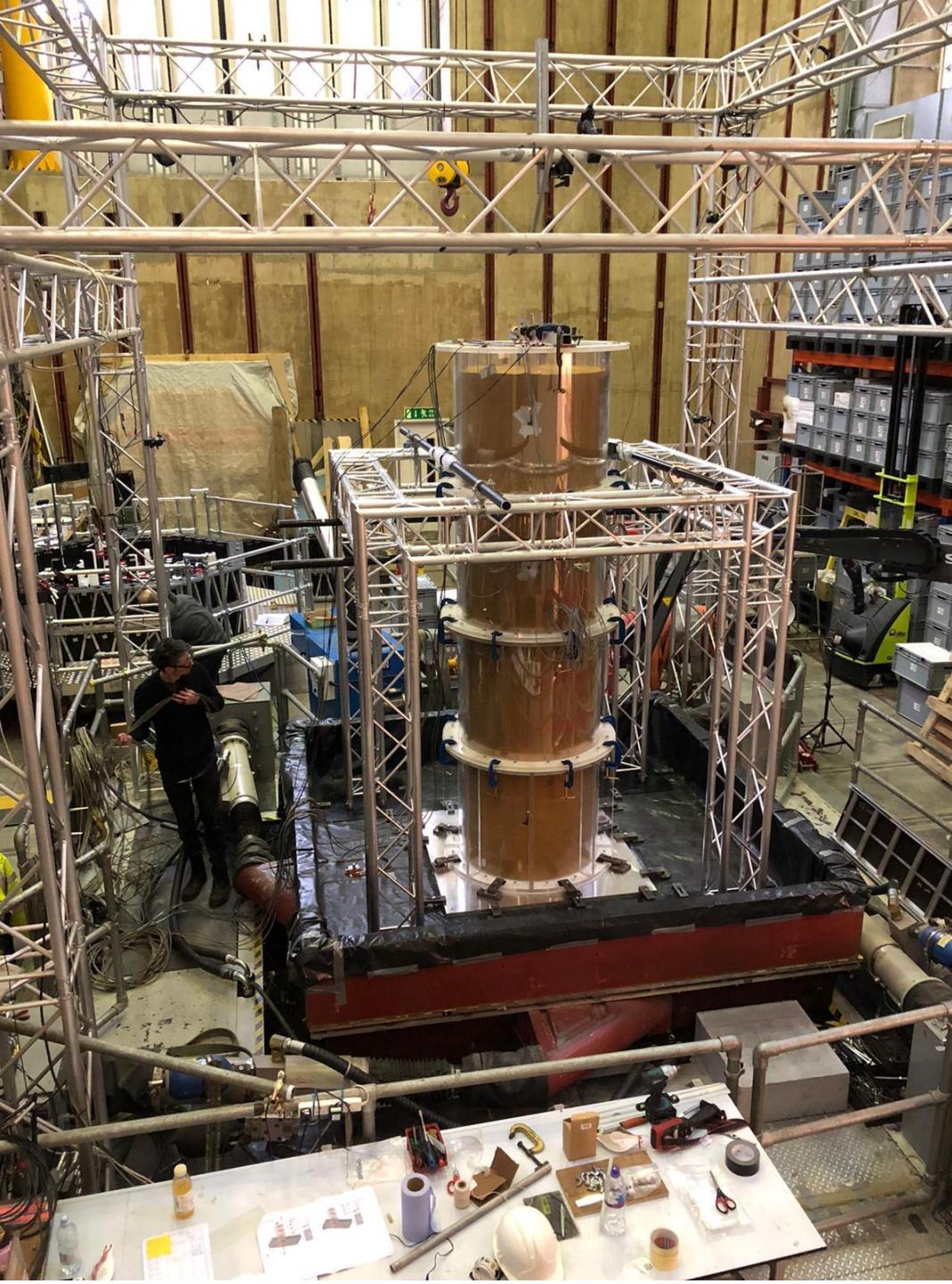


Sketch of the container elements and photo of the whole container located on the table



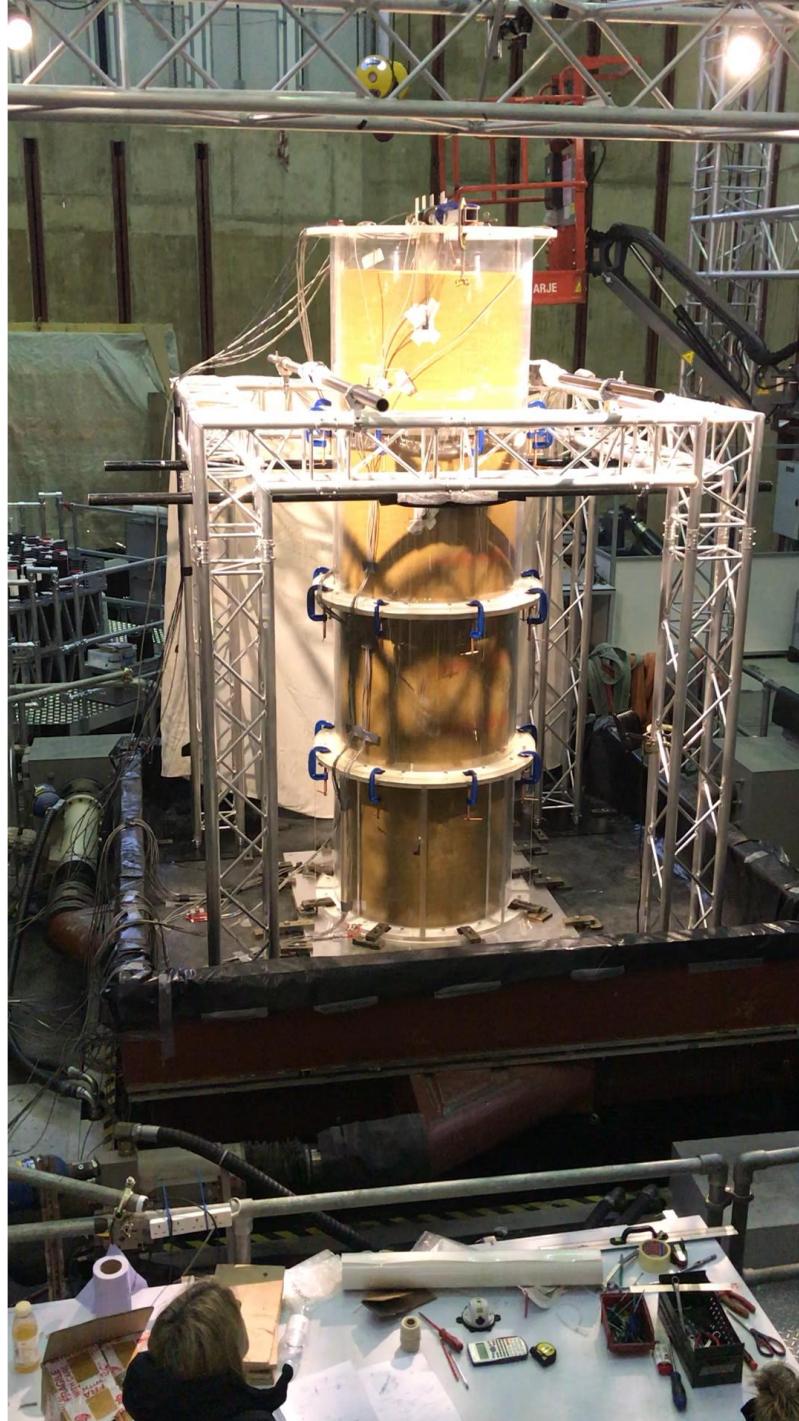














Future effects of such research studies

On the Codes (*... later ?*):

- Data-base of seismic input motions in NF conditions
- Local Seismic Response in NF conditions
- Seismic design actions in Near-Fault areas

On technology developments :

- Seismic base isolation systems in NF conditions
- Soil treatments for reduction of vertical motion amplification



Final Goal

SEISMIC PREVENTION !!!

Prevision of earthquake, alone, is not sufficient !

Further, up to now, we are not very able in seismic prevision ...





Muchas gracias por su atención

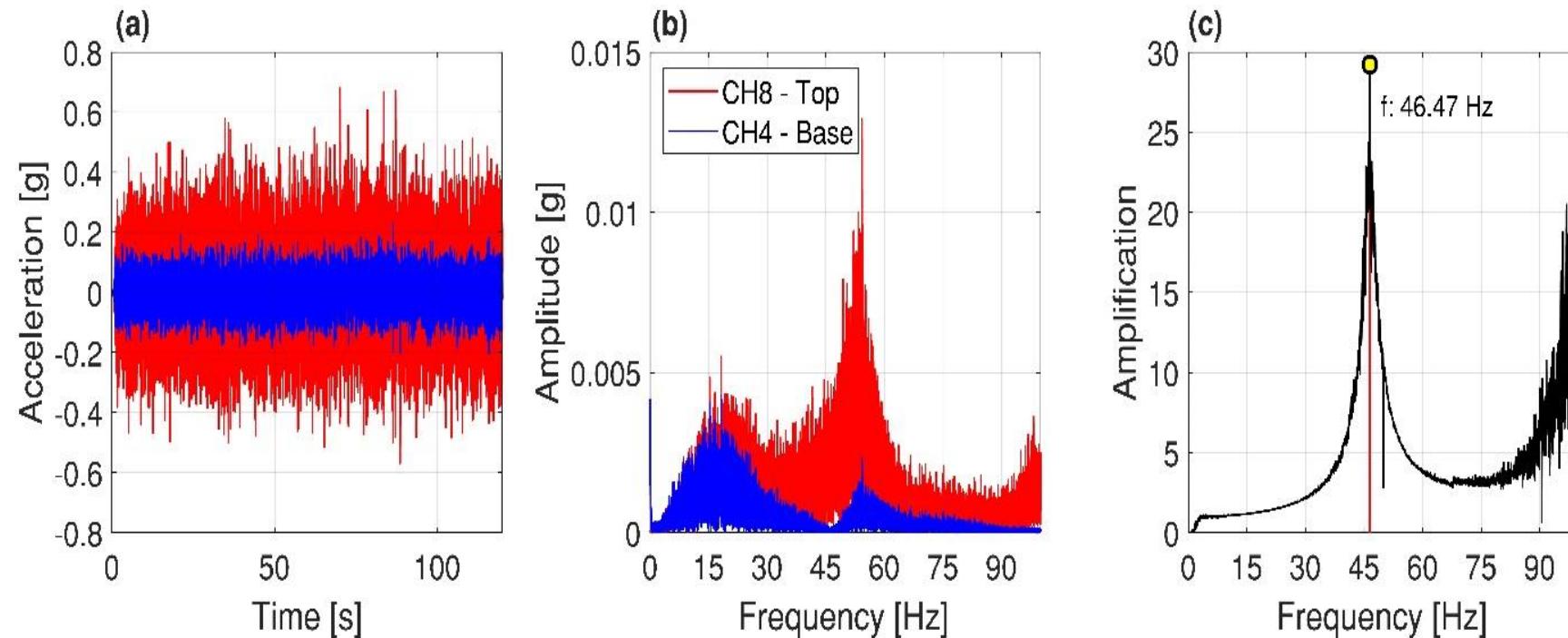




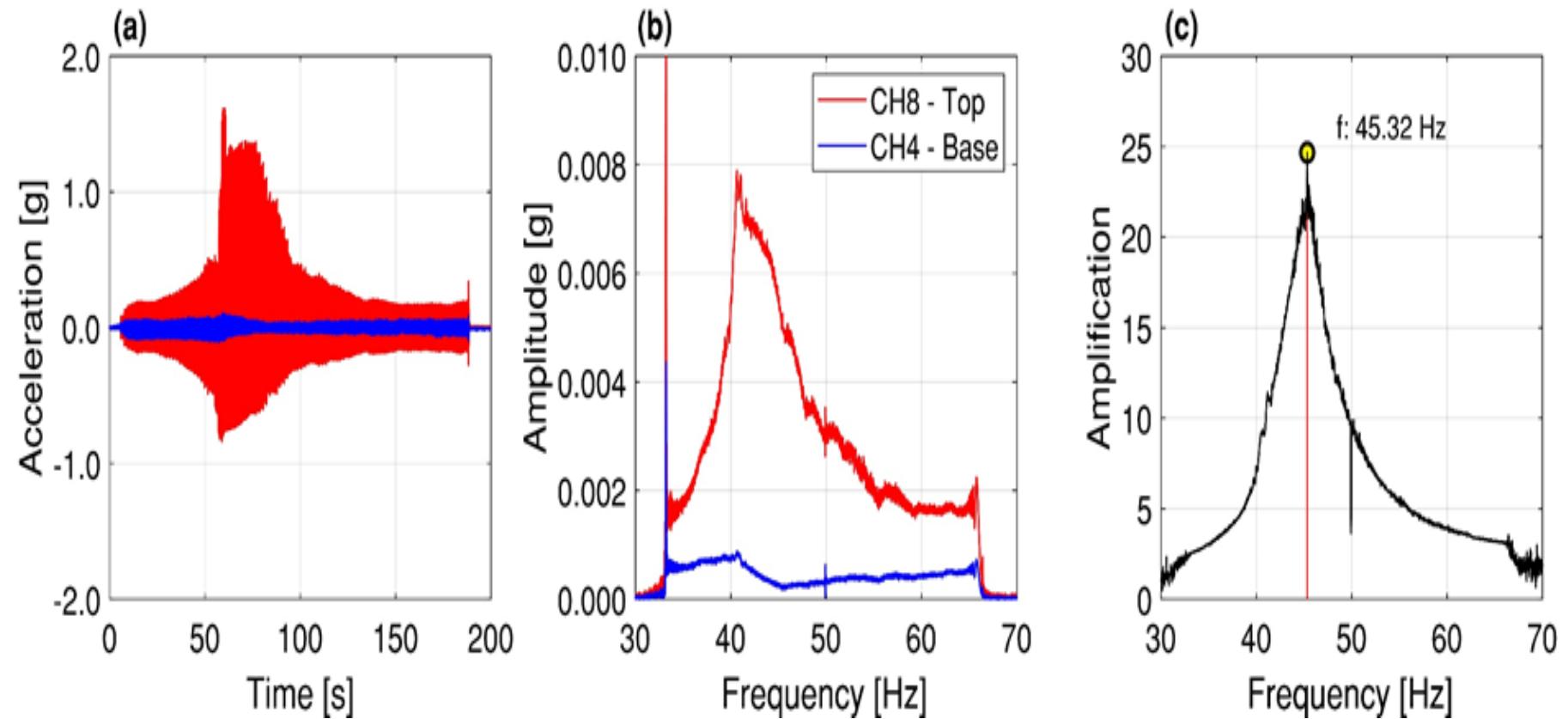
Project SHATTENFEE

Phase 1 – Data Analysis

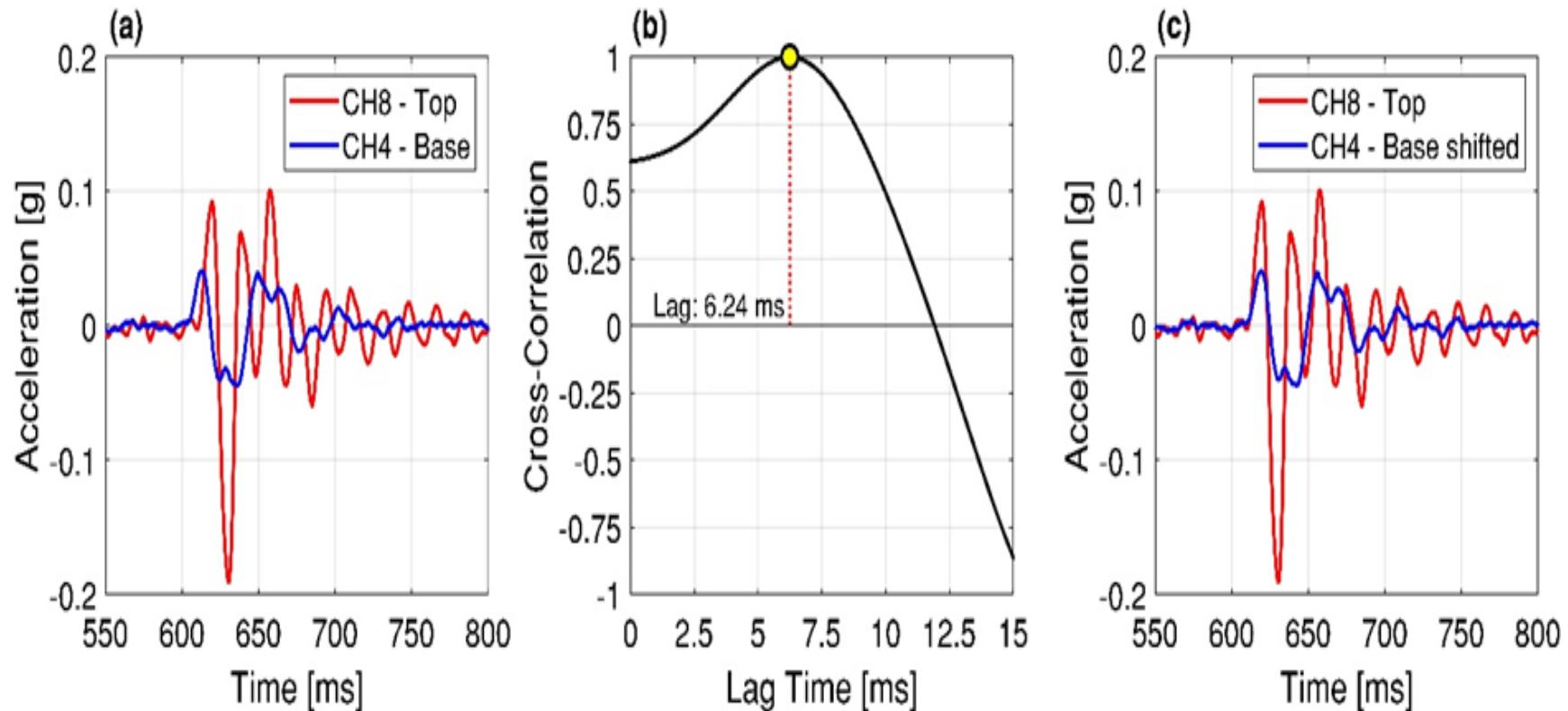
Some preliminar results



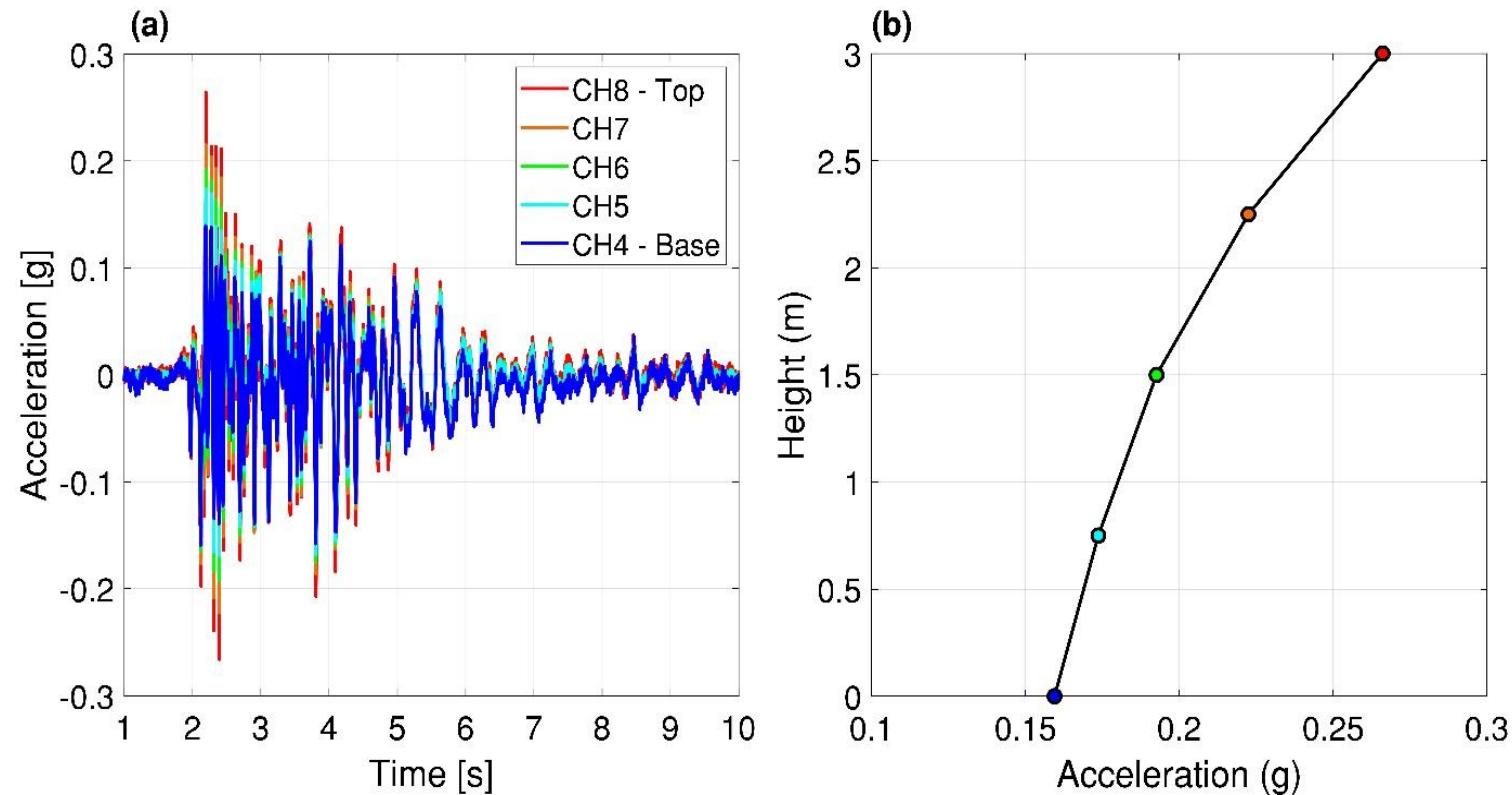
Example of fundamental vertical frequency determination - TEST 25: noise input motion, amplitude 0.05 g, duration 120 sec - (a) Acceleration time-histories at the base (CH4) and top (CH8) of the soil model; (b) Fourier spectra of the two signals; (c) Amplification function and determination of the main frequency f



Example of fundamental vertical frequency determination - TEST 30: sweep function, amplitude 0.05 g, frequency range 33-66 Hz; duration 200 sec - (a) Acceleration time-histories at the base (CH4) and top (CH8) of the soil model; (b) Fourier spectra of the two signals; (c) Amplification function and determination of the main frequency f



Example of compression wave velocity (V_p) measurement – TEST 30: pulse function, amplitude 1 mm; duration 100 sec - (a) Acceleration time-histories at the base (CH4) and top (CH8) of the soil model; (b) Cross-Correlation function and determination of the time delay (travel time); (c) Representation of the shifted acceleration time histories base shifted



Example of vertical acceleration amplification - TEST 45: L'Aquila 2009 earthquake (AQK station, UP component): input accelerogram scaled to a total duration of 20 sec (here only the first 10 sec window is represented)- (a) Acceleration time-histories along the soil height: at the base (CH 4), at intermediate levels (CH. 5, 6 and 7) and on the top of the soil model (CH 8); (b) Maximum acceleration values recorded at each level, from the base to the top